

BUCK JONES WATER LINE REPLACEMENTS

**RALEIGH WATER
RALEIGH, NORTH CAROLINA**

SUBJECT: ADDENDUM NO. 1

FEBRUARY 26, 2024

To the Plans and Specifications for:
Buck Jones Water Line Replacement Project
Raleigh, N.C.

To: PROSPECTIVE PROPOSERS AND OTHER CONCERNED PARTIES

This ADDENDUM forms a part of the Contract Documents and modifies the original Bidding Documents as noted below. Bidders shall acknowledge receipt of the ADDENDUM in the space provided on the Bid Form. Failure to do so may subject the Bidder to Disqualification.

CHANGES TO FRONT END SPECIFICATIONS

Section 00010 – TABLE OF CONTENTS

Page 00010-1 to 00010-2

Remove the section entirely and replace it with Pages 00010-1 (Rev.1) to 00010-2 (Rev.1) included with this addendum.

CHANGES TO TECHNICAL SPECIFICATIONS

Appendices

Add the attached Appendices (Part 1 – Part 3) included with this addendum to the end of the Project Manual.

- | | |
|--------|--|
| Part 1 | NCDEQ Erosion & Sedimentation Control Letter of Approval |
| Part 2 | Geotechnical Investigation Report - Falcon Engineering |
| Part 3 | Vacuum Excavation Test Hole Report - GEL |

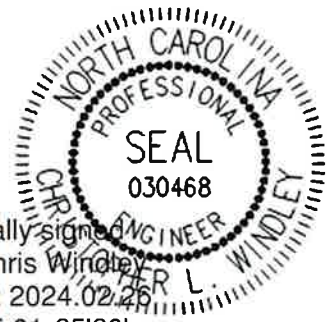
FOR THE OWNER

CJS Conveyance, PLLC

Chris Windley

BY: _____
Christopher L. Windley, P.E.

Digitally signed
by Chris Windley
Date: 2024.02.26
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END OF DOCUMENT

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Part 1 - NCDEQ Erosion & Sedimentation Control Letter of Approval
Part 2 - Geotechnical Investigation Report – Falcon Engineering
Part 3 – Vacuum Excavation Test Hole Report - GEL

END OF DOCUMENT

APPENDIX

* This entire section is project-specific and may include the following attachments for the Contractor's information or use during bidding or construction (for example):

PART 1 NCDEQ Erosion & Sedimentation Control Letter of Approval

PART 2 (see attachment)- Geotechnical Investigation Report - Provided for information purposes only (not part of contract)

PART 3 (see attachment) – Vacuum Excavation Test Hole Report– Provided for information purposes only (not part of contract)

PART-1

ROY COOPER
Governor
ELIZABETH S. BISER
Secretary
DOUGLAS R. ANSEL
Interim Director



August 25, 2023

LETTER OF APPROVAL

City of Raleigh
ATTN: William Wheeler, Director Raleigh Water
PO Box 590
Raleigh, NC 27513

RE: Project Name: Buck Jones Road Area Water Line Replacements
Acres Approved: 6.3
Project ID: WAKE-2024-006
County: Wake
City: Raleigh
Address: Norman Place
River Basin: Neuse
Stream Classification: C;NSW
Submitted By: Chris Windley, PE, CJS Conveyance
Date Received by LQS: July 28, 2023
Plan Type: New/Utility

Dear Mr. Wheeler:

This office has reviewed the subject erosion and sedimentation control plan. We find the plan to be acceptable and hereby issue this Letter of Approval. The enclosed Certificate of Approval must be posted at the job site. This plan shall expire three (3) years following the date of approval, if no land disturbing activity has been undertaken, as is required by Title 15A NCAC 4B .0129.

As of April 1, 2019, all new construction activities are required to complete and submit an electronic Notice of Intent (eNOI) form requesting a Certificate of Coverage (COC) under the NCG010000 Construction General Permit. After the form is reviewed and found to be complete, you will receive a link with payment instructions for the \$100 annual permit fee. After the fee is processed, you will receive the COC via email. As the Financially Responsible Party shown on the FRO form submitted for this project, you **MUST** obtain the COC prior to commencement of any land disturbing activity. The eNOI form may be accessed at deq.nc.gov/NCG01. Please direct questions about the eNOI form to the [Stormwater Program staff](#) in the Raleigh central office. If the owner/operator of this project changes in the future, the new responsible party must obtain a new COC.



Letter of Approval
City of Raleigh
August 25, 2023
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Title 15A NCAC 4B .0118(a) and the NCG01 permit require that the following documentation be kept on file at the job site:

1. The approved E&SC plan as well as any approved deviation.
2. The NCG01 permit and the COC, once it is received.
3. Records of inspections made during the previous 12 months.

Also, this letter gives the notice required by G.S. 113A-61.1(a) of our right of periodic inspection to ensure compliance with the approved plan.

North Carolina's Sedimentation Pollution Control Act is performance-oriented, requiring protection of existing natural resources and adjoining properties. If, following the commencement of this project, it is determined that the erosion and sedimentation control plan is inadequate to meet the requirements of the Sedimentation Pollution Control Act of 1973 (North Carolina General Statute 113A-51 through 66), this office may require revisions to the plan and implementation of the revisions to ensure compliance with the Act.

Acceptance and approval of this plan is conditioned upon your compliance with Federal and State water quality laws, regulations, and rules. In addition, local city or county ordinances or rules may also apply to this land-disturbing activity. This approval does not supersede any other permit or approval.

Please note that this approval is based in part on the accuracy of the information provided in the Financial Responsibility Form, which you provided. You are requested to file an amended form if there is any change in the information included on the form. In addition, it would be helpful if you notify this office of the proposed starting date for this project.

Your cooperation is appreciated.

Sincerely,



Dylan Reinhardt
Assistant Regional Engineer
Land Quality Section

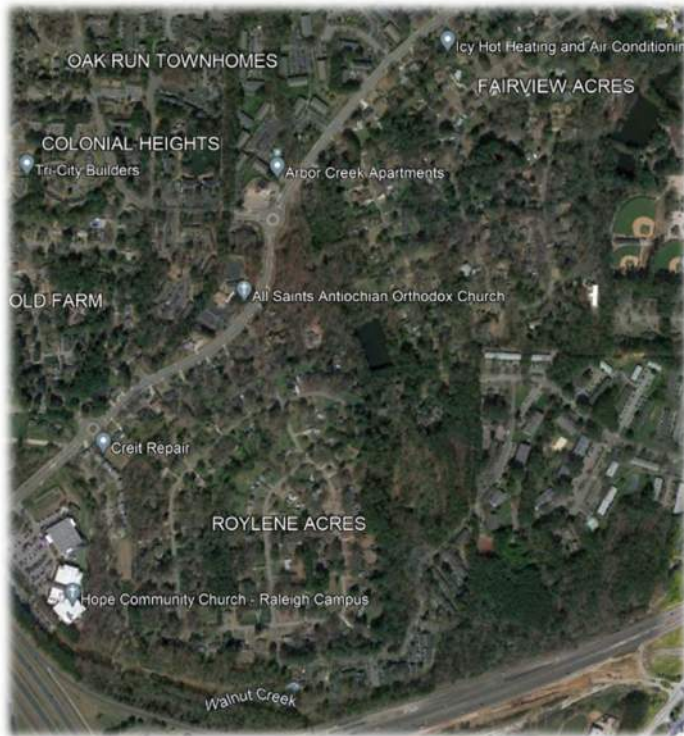
Enclosures: Certificate of Approval
NPDES NCG01 Fact Sheet

cc: Regional Office file
Chris Windley, PE, CJS Conveyance [electronic]

PART-2

GEOTECHNICAL REPORT OF SUBSURFACE INVESTIGATION

BUCK JONES RD AREA WATERLINE REPLACEMENTS RALEIGH, NORTH CAROLINA



PREPARED FOR:
CJS CONVEYANCE, PLLC
320 S. ACADEMY STREET
CARY, NORTH CAROLINA 27511

PREPARED BY:
FALCON ENGINEERING, INC.
1210 TRINITY ROAD, SUITE 110
CARY, NORTH CAROLINA 27513

PROJECT NUMBER: G22029.00
JULY 15, 2022





July 15, 2022

Mr. Alex Biermann
abiermann@cjsconveyance.com
CJS Conveyance, PLLC
320 S. Academy St.
Cary, North Carolina 27511

Re: **Geotechnical Report of Subsurface Investigation**
Buck Jones Rd Area Waterline Replacements
Raleigh, North Carolina
Falcon Project No.: G22029.00

Dear Mr. Biermann:

As authorized, Falcon Engineering, Inc. (Falcon) has completed the subsurface investigation for the above referenced project. This subsurface investigation was conducted in June 2022. The opinions and observations rendered in this report are based solely on our site reconnaissance, performance of twenty-three (23) soil test borings, engineering evaluation of the data obtained, laboratory testing, and generally accepted geotechnical engineering practices and principles. Falcon appreciates the opportunity to have provided geotechnical engineering services to CJS Conveyance, PLLC (CJS) and Raleigh Water (City) for this project. If you have any questions concerning the contents of this report or need additional information, please do not hesitate to contact our office.

Sincerely,
FALCON ENGINEERING, INC.



Allan Paul, PE
Project Manager / Project Engineer

A handwritten signature in black ink, appearing to read "Jeremy R. Hamm".

Jeremy R. Hamm, PE
Geotechnical Services Manager

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SECTION 1: PROJECT INFORMATION

This report presents the field and laboratory test procedures and their results and geotechnical recommendations for the proposed project. Our investigation was performed in general accordance with the scope outlined in our proposal F2021-073 dated September 1, 2021.

1.1 PROJECT DESCRIPTION

Based on our conversations and provided electronic drawings and CAD files, it is our understanding the following apply to the project:

- Raleigh Water has selected CJS to provide engineering and design services for several waterline replacements in the Buck Jones Rd area of Raleigh, North Carolina.
- The project consists of replacing waterlines along Norman Pl, S Lakeside Dr, Jeffries St, Stockton Dr, and Barclay Dr.
- Pipe sizes are anticipated to be on the order of 8 inches in diameter for the majority of the project and consist of ductile iron.
- The project also consists of selective replacement of existing sanitary sewer pipe with new, 8-inch diameter DIP.

Should any of the above information or assumptions made by Falcon be inconsistent with the planned project, we request that you contact us immediately to allow us to make any necessary modifications to this report.

1.2 SITE DESCRIPTION

As depicted on the Site Vicinity Map in Appendix A of this report, the Project Corridor is located in Raleigh, North Carolina. The Project Corridor consists of existing public streets within a predominantly residential area. In addition to pavements, curbs, gutters, sidewalks, underground/overhead utilities, as well as trees are present within the corridors.

1.3 SITE GEOLOGIC DESCRIPTION

According to the *Geologic Map of North Carolina* (1985) and the *Geologic Map of the Cary 7.5-Minute Quadrangle, Wake and Durham Counties, North Carolina* (2016), the work areas are located in one (1) mapped unit of the Piedmont Physiographic Province of North Carolina.

- **Richland Creek Schist (CZrcs)** Silver-gray, white-gray, or tan-white fine to medium grained, layered, and well foliated schist, phyllonite, and fine-grained gneiss.

In addition to the primary mapped unit, thin bands of a secondary unit are present in the corridor:

- **White mica graphite schist (CZwgs)** Silver-gray to black-gray, fine to medium, well-foliated white mica, graphite, biotite (dark mica) and quartz.

1.4 TOPOGRAPHICAL DATA

Terrain models of each proposed work area were provided to us for our use. The terrain models were used to create topographic contour lines of the ground surface, interpolate surface elevations at boring locations, and to generate a ground surface line for subsurface profiles.

SECTION 2: PURPOSE AND SCOPE

Falcon has performed a geotechnical subsurface investigation for the proposed project. The purpose of this investigation is to provide a general characterization of existing onsite soils, rock, and groundwater conditions, earthmoving recommendations, discussion of pipe bedding and haunching, backfill selection and placement recommendations, site excavation considerations, and groundwater considerations.

The project was accomplished through completion of the following tasks:

- Site reconnaissance by Falcon's Geotechnical Engineering personnel.
- Performance of twenty-two (22) Standard Penetration Test (SPT) soil borings and one (1) hand auger boring.
- Visual-manual classification and stratification of the soil samples according to the Unified Soil Classification System (USCS).
- Laboratory testing of select soil samples collected from the borings.
- Analysis of field and laboratory test data and collected soil samples.
- Preparation of this formal engineering report summarizing the field and laboratory test results and our geotechnical recommendations for design and construction.

SECTION 3: FIELD INVESTIGATION

3.1 SITE RECONNAISSANCE AND PROJECT SET-UP

Boring coordinates were selected and provided by CJS in tabular format. Boring coordinates provided were uploaded to a handheld GPS unit capable of sub-meter accuracy and marked in the field. Falcon personnel contacted the North Carolina One-Call Center to request subscriber utilities be located and marked in and around the marked boring locations. Utilities were either marked in the field or noted to be specifically not in conflict with the boring locations prior to the beginning of our field investigation.

3.2 SOIL TEST BORINGS

On June 13th and 14th, 2022, one (1) hand auger boring was advanced approximately 8.0 feet below the existing pavement surface within the Project Corridor. Five (5) additional hand auger borings were attempted but encountered auger refusal before reaching the termination depth of 8.0 feet. Grab samples were collected at changes in strata. In-situ penetration testing was performed via Dynamic Cone Penetrometer (DCP) testing at approximate depths of 2.0, 4.0, and 6.0 feet below the pavement surface. DCP testing was performed in general accordance with ASTM STP 399 “Dynamic Cone for Shallow In-Situ Penetration Testing, Vane Shear and Cone Penetrations Resistance Testing of In-Situ Soils”.

Between June 28th and 30^h, 2022, twenty-two (22) Standard Penetration Test (SPT) soil borings advanced within the Project Corridor including the locations of the five (5) previously attempted hand auger borings. Falcon geotechnical field staff directed drilling at or near the staked/marked boring locations, adjusted as necessary to avoid marked underground utilities, overhead utilities and/or other obstructions at each boring location, and to accommodate traffic control operations. During the field investigation, *as-drilled* boring coordinates were collected with a handheld GPS unit capable of sub-meter accuracy.

SPT borings were advanced to a maximum depth of approximately 10.0 feet below the pavement surface by a CME 55 truck mounted drill rig equipped with hollow stem augers. SPT borings were performed in general accordance with ASTM D1586 “Penetration Test and Split-Barrel Sampling of Soils”. Soil samples were obtained from soil borings at regular intervals using a split-barrel sampler and visually classified in accordance with the Unified Soil Classification System (USCS) by our geotechnical field staff. An automatic hammer was used to advance the sampler at each test interval. Two (2) bulk soil samples were obtained from auger cuttings.

Soil samples were sealed in moisture proof containers, labeled, and transported to our laboratory for further analysis.

SECTION 4: LABORATORY TESTING

All soil samples were reviewed and visually-manually classified in accordance with ASTM D2488 “Standard Practice for Description and Identification of Soils (Visual-Manual Procedure)” and the Unified Soil Classification System (USCS) by our geotechnical staff. During review of the collected soil samples, a number of representative samples were selected for further analysis in Falcon’s soils laboratory.

- Two (2) bulk samples (BS-#) were tested for natural moisture content (ASTM D2216), Atterberg limits (ASTM D4318), mechanical sieve analyses (ASTM D6913), and standard Proctor compaction (ASTM D698).
- Nine (9) split-spoon (SS-#) samples were tested for natural moisture content, Atterberg limits, and mechanical sieve analyses.
- Twelve (12) additional samples were tested for natural moisture content only.

Moisture content testing results are shown on the individual Test Boring Logs in Appendix B of this report. A summary of classification test results and bulk sample test results are shown in the tables below. Detailed soil laboratory testing results can be found in Appendix C of this report.

TABLE 4.1: SUMMARY OF LABORATORY INDEX TESTING

SAMPLE ID	BORING ID	DEPTH (FT)	NATURAL MOISTURE CONTENT (%)	PERCENT PASSING			ATTERBERG LIMITS			USCS SYMBOL
				#10	#40	#200	LL	PL	PI	
SS-02	B-02	1.5-2.0	24.7	94	90	70.1	54	48	6	MH
BS-01	B-03	1.0-6.0	20.3	98	94	71.6	38	38	NP	ML
SS-04	B-05	3.5-5.0	16.2	94	89	67.3	34	34	NP	ML
SS-05	B-06	1.0-2.5	20.7	77	72	51.9	43	23	20	CL
SS-07	B-08	3.5-5.0	15.8	74	68	49.0	42	21	21	SC
SS-09	B-10	3.5-5.0	29.4	89	86	76.6	76	44	32	MH
BS-02	B-13	1.0-5.0	19.2	94	88	61.7	45	25	20	CL
SS-13	B-15	1.0-2.5	12.4	99	94	61.9	21	13	8	CL
SS-15	B-17	1.0-2.5	14.4	81	72	52.8	49	25	24	CL
SS-17	B-19	1.0-2.5	26.0	97	93	80.8	69	33	36	CH
SS-20	B-22	3.5-5.0	25.2	95	90	70.6	55	38	17	MH

TABLE 4.2: SUMMARY OF BULK SAMPLE TESTING

SAMPLE ID	BORING ID	DEPTH (FT)	NATURAL MOISTURE CONTENT (%)	OPTIMUM MOISTURE CONTENT (%)	MAXIMUM DRY UNIT WEIGHT (PCF)	PERCENT PASSING #200	USCS SYMBOL
BS-01	B-03	1.0-6.0	20.3	19.1	104.1	71.6	ML
BS-02	B-13	1.0-5.0	19.2	17.5	110.7	61.7	CL

SECTION 5: SUBSURFACE CONDITIONS

5.1 SURFACE MATERIALS, SOIL, AND ROCK

Surface materials encountered in the borings primarily consist of bituminous concrete (asphalt pavement) underlain by aggregate base course (ABC stone). Asphalt was present at the ground surface in all twenty-three (23) locations explored and each location was underlain by ABC stone. Asphalt and ABC stone thicknesses ranged from 2 to 6 inches and 4 to 16 inches, respectively. Topsoil, concrete sidewalks, and concrete curbs and gutters are present within the Project Corridor as well but were not investigated. Pavement sections may vary in thickness and material type within the Project Corridor.

Fill soils are best described as man-placed deposits of materials used to raise or restore grades that typically include soil and rock but can sometimes consist of trash and debris. Fill was identified in four (4) unique borings extending approximately 2.1 to 3.0 feet below the current road surface. Boring B-06 was terminated at 10 feet below the road surface in fill and the total depth of fill is unknown at this location. We assume fill placed around boring B-06 is associated with construction of the culvert over the stream near this location. Recovered samples of fill were either visually-manually or laboratory classified as silty sand (SM), clayey sand (SC), lean clay (CL), and plastic clay (CH). Relative moisture of fill samples recovered indicates moist conditions. Fill likely exists elsewhere within the corridor(s), extending to various depths, in areas not explored.

Residual soils are formed from the in-place weathering of the parent bedrock. Residual soils were identified below surface materials or existing fill in twenty-two (22) borings. Recovered samples of residuum were either laboratory or visually-manually classified as clayey gravel (GC), silty sand (SM), clayey sand (SC), silt (ML), elastic silt (MH), lean clay (CL), or plastic clay (CH). Relative moisture contents of samples recovered indicates dry to moist conditions.

5.2 GROUNDWATER MEASUREMENTS

After each boring was drilled, the boreholes were inspected for the presence of groundwater, cave-in depths were recorded, and boreholes were backfilled with soil cuttings and asphalt cold patch. Immediate groundwater was not observed in any boring. Borehole cave-in depths are shown on the boring logs. Cave-in depths without observed groundwater may indicate the presence of trapped or static groundwater at or just below the level of cave-in especially where wet or saturated soils are noted on the boring logs.

5.3 VISUALIZATION OF SUBSURFACE DATA

The Legend to Soil and Rock Classification and Symbols can be used as a reference for symbols, common definitions, or other terminology used in textual/graphical representation of subsurface data. The Boring Location Plans were created by overlaying the alignments provided to us in CAD format over publicly available georeferenced aerial imagery. Subsurface conditions at each boring were compiled on to individual Test Boring Logs. Notable subsurface conditions are tabulated on the Summary of Subsurface Materials. These documents can be found in Appendix B of this report.

SECTION 6: GEOTECHNICAL RECOMMENDATIONS AND CONSIDERATIONS

A summary of geotechnical issues with the potential to impact design and construction are provided below. Detailed discussions of each of these issues are provided in the sections that follow.

- **Moisture sensitive soils** are present throughout the alignment. These soils are sensitive to damage from exposure to water. Earthmoving operations performed during the wet winter months (generally November to March) or during periods of inclement weather will be difficult and time consuming.
- **Most existing soils** within the proposed alignments will likely be suitable for use as structural fill/backfill provided they are low plasticity, free of debris, free of particles exceeding 4 inches in the longest dimension and are placed at suitable moisture contents.
- **Additional bedding** may be needed for pipes laid near boring B-06 due to the relatively soft consistency of the existing fill soils.

6.1 EARTHMOVING

6.1.1 STREET CUTS

The Contractor should be prepared to saw cut through asphalt in order to excavate new trenches and restore pavements in accordance with applicable City standards. Pavement materials removed from the alignment should be disposed in accordance with applicable local, state, and federal regulations.

6.1.2 ENGINEERING BEHAVIOR OF SOILS

After pavement removal, the onsite soils will be exposed to weather events. Extended periods of rain or intrusion of runoff may damage otherwise suitable site subgrades necessitating repair or remediation. Excessive degradation of fill soils can be mitigated by compacting near-surface lifts at, or wet of, optimum moisture and achieving at least 98 percent compaction.

Earthmoving operations performed during wet, winter months (November to March) will be difficult and time consuming (i.e., expensive) as it will require maintaining dry and stable excavations and drying of backfill soils. Traditional drying operations will be minimally effective during this time. Trench excavation spoils should be protected from rain events by covering with plastic sheeting or tarps.

6.2 SITE EXCAVATION

6.2.1 GENERAL EXCAVATION

Normal sized earth moving equipment such as rubber-tire backhoe and small to medium sized, track-mounted hydraulic excavators should be suitable to excavate the subsurface materials to the proposed depths.

6.2.2 BOULDERS, PWR, AND ROCK EXCAVATION

We do not anticipate difficult excavation for this project; however, the contract documents should define terms for demonstrating rock and for payment of rock excavation. Typically, a late-model, track-mounted hydraulic excavator, such as a CAT 330 or similar, equipped with rock teeth is used to classify rock excavation. Such a machine would likely be needed to remove PWR encountered in trench, pit, and manhole excavations but would likely struggle to remove material yielding blow counts higher than 50/0.3 blows per foot. Boulders larger than 0.75 cy should also be considered trench rock.

6.2.3 EXCAVATION SAFETY

All excavations deeper than 4 feet must conform to applicable sections of the Construction Industry Occupational Safety and Health Administration (OSHA) Standards (29CFR1926). In general, compliance will require either sloping back excavations or the use of trench boxes or temporary shoring systems, or some combination of both. The referenced (OSHA) standard should be reviewed for requirements regarding use of sloping and/or trench boxes. The shoring system(s) should be designed to resist lateral earth stresses from existing soils and any nearby structures, account for any adjacent roadways or other infrastructure, and include any surcharge loading for construction equipment or public traffic. Designs should include an appropriate hydrostatic pressure to account for rises in groundwater levels and/or water infiltrating the retained soils. The selected system should consider this condition and the design should address feasible penetration depth. Subsurface conditions, depth of excavations, and horizontal and vertical space constraints will dictate the design of the shoring system along with other considerations such as local availability of materials and equipment. It is the contractor's responsibility to design and construct stable, temporary excavations as part of their safety procedure in accordance with local, state, and federal safety regulations. Falcon does not assume responsibility for construction safety or the contractor's or other party's compliance with applicable safety or other regulations. In addition to OSHA standards, the Contractor's excavation safety plan should comply with any City standards and NCDOT requirements within an encroachment agreement. These agencies may require the use of active shoring (i.e. no trench boxes) especially in larger excavations.

6.3 FILL SELECTION, PLACEMENT, AND COMPACTION

6.3.1 MATERIAL SELECTION

Most of the soils encountered in the borings meet the requirements for Class II and Class IV soils classifications for bedding and backfilling per City standards. If rock fragments greater than 6 inches are encountered during excavation, they should be separated from the stockpile and removed from the site. If rock fragments between 2 and 6 inches in size are encountered during excavation, they should be separated from the initial backfill soils so that they are not inadvertently placed immediately against the new pipe. This material may be used in general trench backfill after initial backfilling is complete.

6.3.2 PIPE BEDDING

Per City standards/details, ductile iron pipe may be placed on flat trench bottoms with the pipe resting on stable, undisturbed earth. We anticipate that 6 inches of #67 stone may be needed for pipe bedding from station 19+50 to station 22+50 for both water and sewer pipes placed in this area in order to stabilize the relatively soft soil encountered in the nearby boring.

6.3.3 HAUNCHING, INITIAL BACKFILL, AND FINAL BACKFILL

We take no exception to the City standards/details for haunching, initial backfill and final backfill for pipes on this project.

6.3.4 PLACEMENT AND COMPACTION

Fill and backfill operations should be continuously monitored and documented. Per City standards/details, fill and backfill should be placed in 6-inch loose lifts (or thinner). We recommend using hand-guided compaction equipment in close proximity to the pipe so as to avoid damaging pipes. Fill and backfill should be placed and compacted to a uniform, maximum dry density as noted in accordance with the following:

TABLE 6.1: FILL AND BACKFILL REQUIREMENTS

LOCATION	MATERIAL RESTRICTIONS	COMPACTION REQUIREMENTS	MOISTURE REQUIREMENTS
Backfill beneath pavements	Class I, Class II, Class III, and Class IV soils per City standards	95% of maximum dry unit weight per ASTM D698/AASHTO T99	±2% of optimum moisture
Backfill outside of pavements	Class I, Class II, Class III, and Class IV soils per City standards	90% of maximum dry unit weight per ASTM D698/AASHTO T99	±2% of optimum moisture

Soil compaction should be tested in accordance with the sand cone, drive tube, or nuclear density gauge methods at the following minimum frequencies:

TABLE 6.2: MINIMUM COMPACTION TESTING FREQUENCIES

LOCATION	RESTRICTIONS
Trench backfill	One (1) test per lift, per 300 linear feet of backfill placed, per trench, per day

6.4 GROUNDWATER, DEWATERING, DRAINAGE

6.4.1 GROUNDWATER MECHANICS

Groundwater typically flows in the direction of surface water. Groundwater levels will vary with environmental variations and seasonal conditions, such as the frequency and magnitude of rainfall. Consequently, excavations performed during the drier months of the year may yield more favorable groundwater conditions.

Groundwater was not observed in any open borehole immediately after drilling; however, trapped or perched groundwater may be present within existing fill soils especially in low-lying areas. The contractor should be prepared to control groundwater percolating into excavations by using mud pumps or submersible pumps.

6.4.2 SITE DRAINAGE, DIVERSION, AND DEWATERING

Most trenches will be excavated through existing pavements. Stormwater runoff may flow toward excavations in some areas. The Contractor should be prepared to sequence operations such that trenches are backfilled prior to rain events and/or maintain slightly sloping backfill to provide drainage toward sump areas. Surface water flows and any seeping groundwater in excavations can be intercepted by digging sump pits and placing submersible pumps surrounded by gravel to dewater excavations.

6.5 PAVEMENT SUBGRADE AND PATCHING

Per City standards/details, 3.0 inches of new surface course asphalt shall be underlain by a minimum of 12 inches of ABC stone. For temporary backfilling prior to paving, the upper 3 inches of the trench may be backfilled with ABC stone level with the surrounding pavement surface or up to 1 inch higher than the surrounding surface. We take no exception to the City’s Standard Asphalt Pavement Patch Detail (drawing S-3) for this project.

SECTION 7: ADDITIONAL SERVICES

This Geotechnical Subsurface Investigation is intended to be a design level report. Therefore, as the project progresses through the remaining design phase, bidding, and construction, we would be pleased to provide a cost proposal to you in order to perform any or all of the following additional tasks:

- Provide geotechnical consulting services during the remaining design phase and during bidding.
- Review relevant portions of the plans and specifications for compliance with this report.
- Provide Construction Materials Testing (CMT) services during the construction phase of the project.

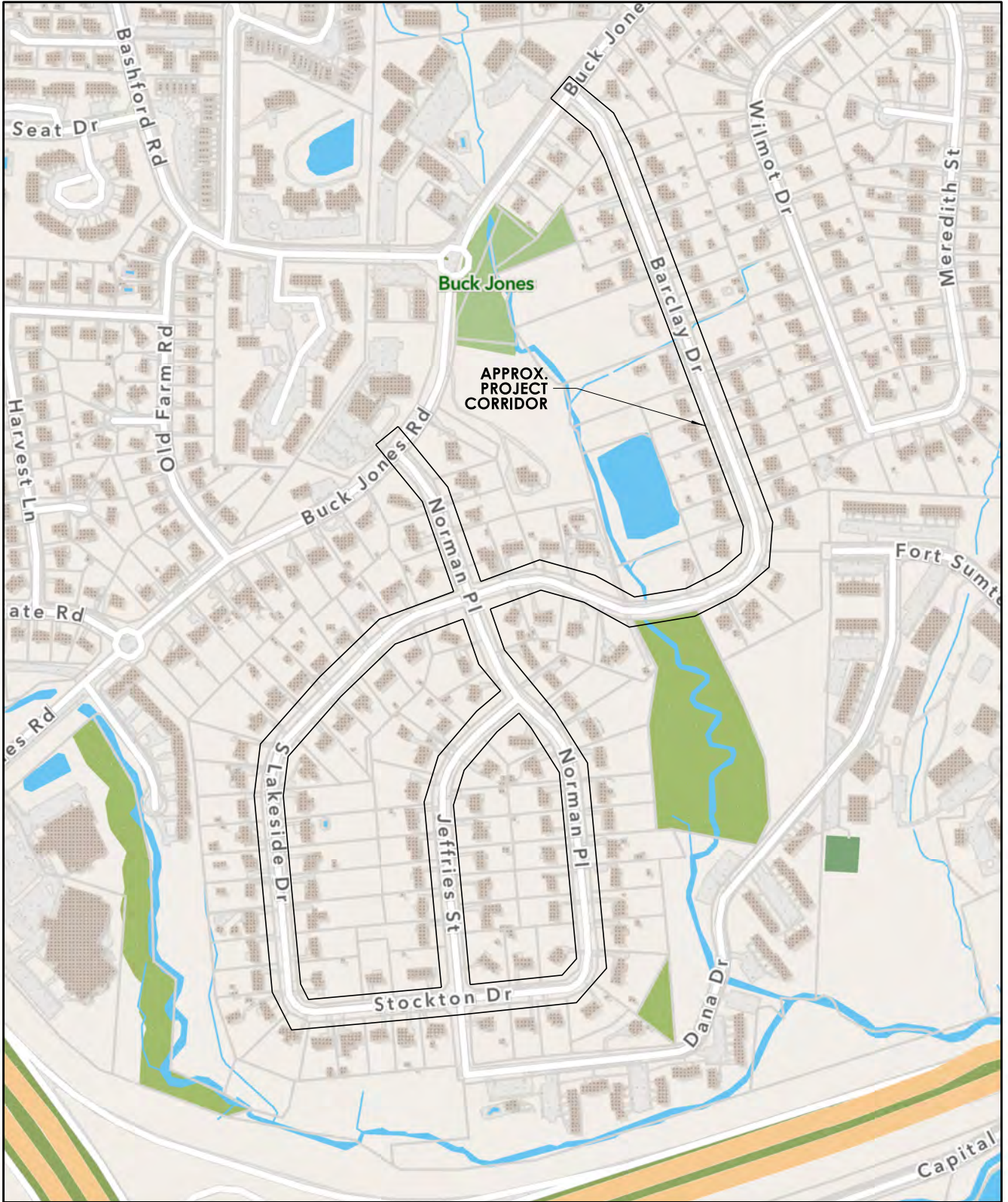
SECTION 8: CLOSURE

Recommendations and evaluations provided by Falcon are based on the project description as outlined herein. Modifications of our recommendations and evaluations may be required if there are changes to the proposed project. Recommendations in this report are based on data obtained from our subsurface field exploration and laboratory testing programs. The nature and extent of variations between borings may not become evident until construction, although more insight may be provided by additional field testing data.

Our professional services for this project have been performed in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made. Falcon appreciates this opportunity to have provided you with geotechnical engineering services for this project. If you have any questions regarding this report, please contact our office at 919.871.0800.

APPENDIX A

SITE VICINITY MAP A-1



SHEET: SITE VICINITY MAP

PROJECT NAME: BUCK JONES RD WATERLINE REPLACEMENTS

PROJECT NO.: G22029.00

PROJECT LOCATION: RALEIGH, NORTH CAROLINA

HORIZONTAL SCALE 1"=500'

INVESTIGATED BY: RWL

DRAWN BY: ASP

CHECKED BY: JRH

DATE: 2022-07-11



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APPENDIX B

LEGEND TO SOIL AND ROCK CLASSIFICATIONS AND SYMBOLS	B-1
BORING LOCATION PLAN	B-2
TEST BORING LOGS.....	B-3
SUMMARY OF SUBSURFACE MATERIALS	B-4

LEGEND TO SOIL AND ROCK CLASSIFICATION AND SYMBOLS

UNIFIED SOIL CLASSIFICATION SYSTEM (USCS)	FRACTURE SPACING	CONSISTENCY OF COHESIVE SOILS		RELATIVE DENSITY OF COHESIONLESS SOILS
SYMBOLS	TERM	TERM	CONSISTENCY	RELATIVE DENSITY
BITUMINOUS CONCRETE (ASPHALT)	VERY WIDE	MORE THAN 10 FEET	VERY SOFT	VERY LOOSE
CONCRETE	WIDE	3 TO 10 FEET	SOFT	LOOSE
AGGREGATE BASE COURSE	MODERATELY CLOSE	1 TO 3 FEET	FIRM	MEDIUM DENSE
TOPSOIL	CLOSE	0.16 TO 1 FEET	STIFF	DENSE
LOW-PLASTICITY ORGANIC SILT/CLAY (OL)	VERY CLOSE	LESS THAN 0.16 FEET	VERY STIFF	VERY DENSE
HIGH-PLASTICITY ORGANIC SILT/CLAY (OH)			HARD	
PEAT (PT)			VERY HARD	
WELL-GRADED GRAVEL (GW)				
POORLY-GRADED GRAVEL (GP)				
SILTY GRAVEL (GM)				
CLAYEY GRAVEL (GC)				
WELL-GRADED SAND (SW)				
POORLY-GRADED SAND (SP)				
SILTY SAND (SM)				
CLAYEY SAND (SC)				
SILT (ML)				
ELASTIC SILT (MH)				
LEAN CLAY (CL)				
PLASTIC CLAY (CH)				
PARTIALLY WEATHERED ROCK				
NON-CRYSTALLINE ROCK				
CRYSTALLINE ROCK				
COASTAL PLAIN SEDIMENTARY ROCK				
MAN PLACED FILL OR BACKFILL				
ALLUVIAL SOILS				
IMMEDIATE WATER LEVEL				
STATIC WATER LEVEL				
PIPE INVERT ELEVATION				
AUGER PROBING				
SPT BORING				
SPT BORING WITH ROCK CORE				
CONE PENETRATION TEST SOUNDING				
HAND AUGER + TEST				
ROD SOUNDING				
TEST PIT				
SPT N-VALUE				
SS SPLIT SPOON SAMPLE				
BS BULK SAMPLE				
ST SHELBY TUBE SAMPLE				
RS ROCK SAMPLE				

WEATHERING

FRESH	Rock fresh, crystals bright, few joints may show slight staining. Rock rings under hammer if crystalline.
VERY SLIGHT (V. SL.)	Rock generally fresh, joints stained, some joints may show thin clay coatings if open, crystals on a broken specimens face shine brightly. Rock rings under hammer blows if of a crystalline nature.
SLIGHT (SLI.)	Rock generally fresh, joints stained and discoloration extends into rock up to 1 inch. Open joints may contain may contain clay. In granitoid rocks some occasional feldspar crystals are dull and discolored. Crystalline rock rings under hammer blows.
MODERATE (MOD.)	Significant portions of rock shows discoloration and weathering effects. In granitoid rocks, most feldspars are dull and discolored, some show clay. Rock has dull sound under hammer blows and show significant loss of strength as compared with fresh rock.
MODERATE SEVERE (MOD. SEV.)	All rocks except quartz discolored or stained. In granitoid rocks, all feldspars and discolored and a majority show kaolinitization. Rocks shows severe loss of strength and can be excavated with a geologist's pick. Rock gives "clunk" sound when struck. If tested would yield spt refusal.
SEVERE (SEV.)	All rocks except quartz discolored or stained. Rock fabric clear and evident but reduced in strength to strong soil. In granitoid rocks all feldspars are kaolinitized to some extent. Some fragments of strong rock usually remain. If tested, yields spt n-values > 100 bpf.
VERY SEVERE (V. SEV.)	All rocks except quartz discolored or stained. Rock fabric elements are discernible but the mass is effectively reduced to soil status, with only fragments of strong rock remaining. Saprolite is an example of rock weathered to a degree such that only minor vestiges of the original rock fabric remain. If tested, yields spt n-values < 100 bpf.
COMPLETE	Rock reduced to soil. Rock fabric not discernible or discernible only in small and scattered concentrations. Quartz may be present as dikes or stringers. Saprolite is also an example.

ROCK HARDNESS

VERY HARD	Cannot be scratched by knife or sharp pick. Breaking of hand specimens requires several hard blows of the geologist's pick.
HARD	Can be scratched by knife or pick only with difficulty. Hard hammer blows required to detach hand specimens.
MODERATELY HARD	Can be scratched by knife or pick. Gouges or grooves to 0.25 inches deep can be excavated by hard blow of a geologist's pick. Hand specimens can be detached with moderate blows.
MEDIUM HARD	Can be grooved or gouged 0.5 inches deep by firm pressure of knife or pick point. Can be excavated in small chips to pieces 1 inch maximum size by hard blows of the point of a geologist's pick.
SOFT	Can be grooved or gouged readily by knife or pick. Can be excavated in fragments from chips to several inches in size by moderate blows of a pick point. Small, thin pieces can be broken by finger pressure.
VERY SOFT	Can be carved with knife. Can be excavated readily with point of pick. Pieces 1 inch or more in thickness can be broken by finger pressure. Can be scratched readily by fingernail.

ROCK DEFINITION

Hard rock is non-coastal plain material that when tested, would yield spt refusal. An inferred rock line indicates the level at which non-coastal plain material would yield SPT refusal. SPT refusal is penetration by a split-spoon sampler equal to or less than 0.1 foot per 50 blows. In non-coastal plain material, the transition between soil and rock materials are typically divided as follows:

PARTIALLY WEATHERED ROCK (PWR)		Non-coastal plain material that yields SPT N-values > 100 blows per foot.
CRYSTALLINE ROCK (CR)		Fine to coarse grained, igneous and metamorphic rock that would yield SPT refusal if tested. Rock type includes granite, gneiss, gabbro, schist, etc.
NON-CRYSTALLINE ROCK (NCR)		Fine to coarse grained, metamorphic and non-coastal plain sedimentary rock that would yield SPT refusal if tested. Rock type includes phyllite, slate, sandstone, etc.
COASTAL PLAIN SEDIMENTARY ROCK (CP)		Coastal plain sediments cemented tinto rock but may not yield SPT refusal. Rocky type includes limestone, sandstone, cemented shell beds, etc.

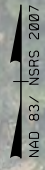
ABBREVIATIONS

ABC	Aggregate base course	FIAD	Filled immediately after drilling	RES	Residuum
ALLUV	Alluvium	FOSS	Fossiliferous	SAP	Saprolitic
AR	Auger refusal	FRAC	Fractured	S	Soft
BC	Bituminous concrete (asphalt)	FRAGS	Fragments	SAT	Saturated
BLDR	Boulder	GR	Gravel	SD	Sand
BPF	Blows per foot	GS	Specific gravity	SDY	Sandy
BT	Boring terminated	GW	Groundwater	SED	Sediments
CALC	Calcareous	HR	Hard rock	SL	Silt, silty
CI	Caved-in	MED	Medium	SLI	Slightly
CL	Clay	MIC	Micaceous	SPT	Standard penetration test
CLY	Clayey	MOT	Mottled	SWR	Soft weathered rock
COB	Cobble	NS	No sample taken	TCR	Tricone refusal
CSE	Coarse	ORG	Organic	TS	Topsail
DPT	Dynamic penetration test	PP	Pocket penetrometer	VST	Vane shear test
DST	Estimated	PP	Pocket penetrometer	V	Very
F	Fine	PWR	Partially weathered rock	W	With
		REF	Refusal		

TERMS AND DEFINITIONS

- ALLUVIUM: soils which have been transported and deposited by water.
- AQUIFER: a water bearing formation or strata.
- ARENACEOUS: applied to rocks that have been derived from sand or that contain sand. Argillaceous: applied to all rocks or substances composed of clay minerals, or having a notable proportion of clay in their composition, as shale/slate/etc.
- ARTESIAN: groundwater that is under sufficient pressure to rise above the level at which it is encountered, but which does not necessarily rise to or above the ground surface
- CALCAREOUS: soils which contain appreciable amounts of calcium carbonate.
- COLLUVIUM: rock fragments mixed with soil deposited by gravity on a slope or bottom of a slope.
- CORE RECOVERY: total length of all material recovered in the core barrel divided by total length of core run and expressed as a percentage.
- DIKE: a tabular body of igneous rock that cuts across the structure of adjacent rocks or cuts massive rock.
- DIP: the angle at which a stratum or any planar feature is inclined from the horizontal.
- DIP DIRECTION: the direction or bearing of the horizontal trace of the line of dip, measured clockwise from north.
- FAULT: a fracture or fracture zone along which there has been displacement of the sides relative to one another parallel to the fracture.
- FILL: man-made deposits of natural soils or rock products and waste materials.
- FISSLE: a property of splitting along closely spaced parallel planes.
- FLOAT: rock fragments on surface near their original position and dislodged from parent material.
- FLOOD PLAIN: land bordering a stream, built of sediments deposited by the stream.
- FORMATION: a mappable geologic unit that can be recognized and traced in the field.
- JOINT: fracture in rock along which no appreciable movement has occurred.
- LEDGE: a shelf-like ridge or projection of rock whose thickness is small compared to its lateral extent.
- LENS: a body of soil or rock that thins out in one or more directions.
- MOTTLED: irregularly marked with spots of different colors. Mottling in soils usually indicates poor aeration and lack of good drainage.
- PERCHED WATER: water maintained above the normal groundwater level by the presence of an intervening impervious stratum.
- RESIDUUM: soil formed in place by weathering of the parent rock.
- ROCK QUALITY DESIGNATION (ROD): a measure of rock quality described by: total length of rock segments equal to or greater than 4 inches divided by the total length of core run and expressed as a percentage.
- SAPROLITE: residual soil which retains the relic structure or fabric of the parent rock.
- SILL: an intrusive body of igneous rock of approximately uniform thickness and relatively thin compared with its lateral extent, which has been emplaced parallel to the bedding or schistosity of the intruded rocks.
- SLICKENSIDE: polished and striated surface that results from friction along a fault or slip plane.
- STANDARD PENETRATION TEST (SPT): number of blows of a 140 pound hammer falling 30 inches required to produce a penetration of 1 foot (N-value or blows per foot) into soil with a 2 in outside diameter split spoon sampler. SPT refusal is less than 0.1 foot penetration with 50 blows.
- STRATA CORE RECOVERY: total length of strata material recovered divided by total length of stratum and expressed as a percentage.
- STRATA ROCK QUALITY DESIGNATION: a measure of rock quality described by total length of rock segments within a stratum equal to or greater than 4 inches divided by the total length of strata and expressed as a percentage.
- STRATUM: a section of a formation consisting of the same kind of material throughout.
- TOPSOIL: surface soils usually containing organic material.

NOTES:
 1. Georeferenced aerial imagery obtained from www.NCOneMap.gov



SHEET: BORING LOCATION PLAN		INVESTIGATED BY: JH/RL	
PROJECT NAME: BUCK JONES RD WATERLINE REPLACEMENTS		DRAWN BY: ASP	
PROJECT NO.: G22029.00	HORIZONTAL SCALE 1"=200'	CHECKED BY: JRH	
PROJECT LOCATION: RALEIGH, NORTH CAROLINA		DATE: 2022-07-14	



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NOTES:

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PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina		LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC							
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING NO. B-01		BORING LOCATION 0+68 1 ft Right 1		HOLE	DEPTH	FIAD							
ELEVATION (ft) 416.4		NORTHING (ft) 738420		DRILL MACHINE CME 55 TRUCK		DATE 6/28/2022									
TOTAL DEPTH (ft) 10.0		EASTING (ft) 2079826		DRILLER Campbell, T.		SURFACE WATER DEPTH (ft) N/A									
DATE STARTED 6/28/2022		DATE COMPLETED 6/28/2022		DRILL METHOD Hollow Augers		HAMMER TYPE Automatic									
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB. NO. DEPTH LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80						100
												416.4		0.0	
	1.0	3	5	7								416.3	BITUMINOUS CONCRETE 2 inches	0.2	
												415.7	AGGREGATE BASE COURSE 7 inches	0.8	
415										15.4%	SS-01		RESIDUUM Red, SILT (ML) with Mica		
	3.5	5	5	5						Dry					
	6.0	2	2	7								410.9	Red - Orange, Sandy, SILT (ML) with Mica	5.5	
410										Moist					
	8.5	2	4	5						Moist					
												406.4		10.0	
													Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM		
405															
400															
395															

01 TEST BORING LOG: G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Lane, R.		GROUND WATER	0 HOUR	STATIC							
PROJECT NAME Buck Jones Rd Waterline Replacements							HOLE	Dry	FIAD							
BORING NO. B-02		BORING LOCATION 4+40 0 ft 1					DEPTH									
ELEVATION (ft) 425.0		NORTHING (ft) 738104		DRILL MACHINE		DATE		6/13/2022								
TOTAL DEPTH (ft) 8.0		EASTING (ft) 2080004		DRILLER		SURFACE WATER DEPTH (ft) N/A										
DATE STARTED 6/13/2022			DATE COMPLETED 6/13/2022		DRILL METHOD Hand Auger		HAMMER TYPE 15 lb Hammer									
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			AVERAGE BLOWS, Nc					MOISTURE	LAB. NO.	DEPTH	L G O G	Elev. (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)
		1.75 in	1.75 in	1.75 in	0	5	10	15	20							
														425.0		0.0
														424.8	BITUMINOUS CONCRETE 2 inches	0.2
														424.2	AGGREGATE BASE COURSE 8 inches	0.8
	2.0	13	18	15						24.7%	S-02				RESIDUUM Red, ELASTIC SILT (MH) with Mica	
	4.0	14	15	16						Dry						
420	6.0	13	15	17						Dry				419.0	Red, Sandy, SILT (ML) with Mica	6.0
														417.0		8.0
415															Hand Auger Terminated at 8.0 feet Below Current Ground Surface in RESIDUUM	
410																
405																

05 DCP LOG G22029.00.GPJ FALCON FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina		LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC								
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 8+39 3 ft Left 1				HOLE	Dry	FIAD								
BORING NO. B-03		BORING LOCATION 8+39 3 ft Left 1				DEPTH										
ELEVATION (ft) 410.2		NORTHING (ft) 737730		DRILL MACHINE CME 55 TRUCK		DATE	6/28/2022									
TOTAL DEPTH (ft) 10.0		EASTING (ft) 2080143		DRILLER Campbell, T.		SURFACE WATER DEPTH (ft) N/A										
DATE STARTED 6/28/2022		DATE COMPLETED 6/28/2022		DRILL METHOD Hollow Augers		HAMMER TYPE Automatic										
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB NO.	DEPTH LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80							100
410	1.1	3	4	7							Dry			410.2		0.0
														409.9	BITUMINOUS CONCRETE 3 inches	0.3
														409.1	AGGREGATE BASE COURSE 9 inches	1.1
															RESIDUUM Red and Gray, Sandy, SILT (ML) with Mica	
	3.5	4	5	6							Moist	BS-01				
405	6.0	2	4	5							Moist					
	8.5	3	8	11							Moist			402.2	Red and Black, Sandy, SILT (ML) with Mica	8.0
400														400.2		10.0
															Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM	

01 TEST BORING LOG G22029.00.GPJ FALCON FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina		LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC							
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 12+32 1 ft Right 1				HOLE	Dry	FIAD							
BORING NO. B-04		ELEVATION (ft) 410.5		NORTHING (ft) 737363		DEPTH									
TOTAL DEPTH (ft) 10.0		DATE STARTED 6/28/2022		DATE COMPLETED 6/28/2022		DRILL MACHINE CME 55 TRUCK		DATE 6/28/2022							
DATE STARTED 6/28/2022		DATE COMPLETED 6/28/2022		DRILLER Campbell, T.		SURFACE WATER DEPTH (ft) N/A									
DATE STARTED 6/28/2022		DATE COMPLETED 6/28/2022		DRILL METHOD Hollow Augers		HAMMER TYPE Automatic									
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB NO. DEPTH LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80						100
410	1.0	3	5	8							23.4%	SS-03	410.5		0.0
													410.4	BITUMINOUS CONCRETE 2 inches	0.2
													409.7	AGGREGATE BASE COURSE 6 inches	0.8
	3.5	3	6	7									407.5	RESIDUUM Red, Sandy, SILT (ML) with Rock Fragments, Mica	3.0
	6.0	3	4	8							Moist			Red - Brown, Sandy, LEAN CLAY (CL) with Mica	
405	8.5	3	5	6							Moist		405.0	Red - Brown and Gray, Sandy, SILT (ML) with Mica	5.5
											Moist				
400													400.5	Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM	10.0

01 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



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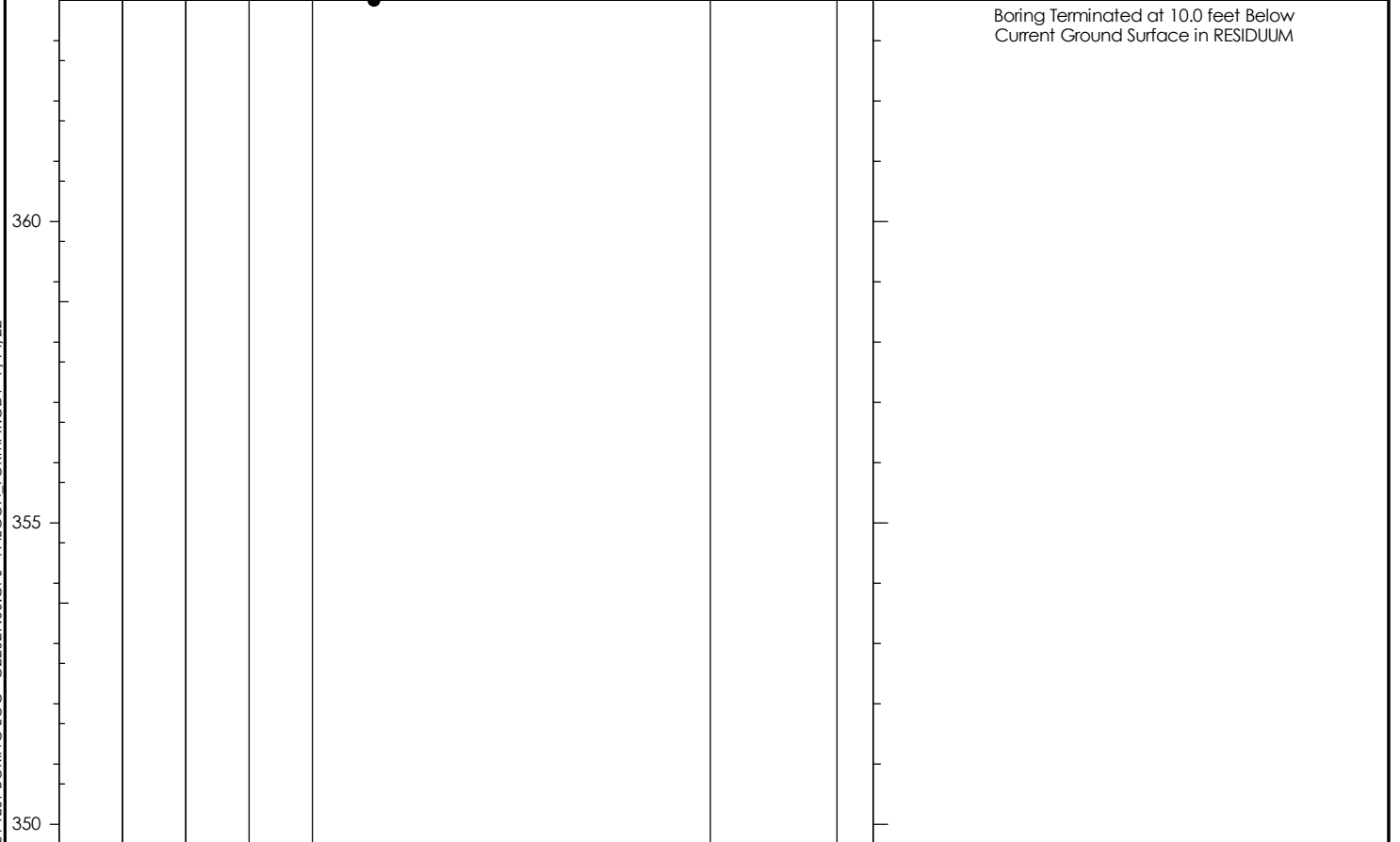
PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC							
PROJECT NAME Buck Jones Rd Waterline Replacements							HOLE	Dry	FIAD							
BORING NO. B-05		BORING LOCATION 16+49 1 ft Left 1					DEPTH									
ELEVATION (ft) 404.0		NORTHING (ft) 736974		DRILL MACHINE CME 55 TRUCK		DATE		6/28/2022								
TOTAL DEPTH (ft) 10.0		EASTING (ft) 2080432		DRILLER Campbell, T.		SURFACE WATER DEPTH (ft) N/A										
DATE STARTED 6/28/2022			DATE COMPLETED 6/28/2022		DRILL METHOD Hollow Augers		HAMMER TYPE Automatic									
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB. NO. / DEPTH	LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION		Depth (ft)
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80					100	GROUNDWATER READINGS	
	1.0													404.0		0.0
														403.7	BITUMINOUS CONCRETE 3 inches	0.3
														403.0	AGGREGATE BASE COURSE 8 inches	1.0
	3.5	3	4	8							Moist				RESIDUUM Red, LEAN CLAY (CL) with Rock Fragments, Mica	
400		6	6	8							16.2%	SS-04		399.5	Tan - Brown, Sandy, SILT (ML) with Mica	4.5
	6.0	4	6	9							Moist					
	8.5	5	9	12							Moist					
395											Moist			394.0		10.0
															Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM	

01 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO.	PROJECT LOCATION	LOGGED BY	GROUND WATER	O HOUR	STATIC
G22029.00	Raleigh, North Carolina	Hill, J.	HOLE	Dry	FIAD
BORING NO.	BORING LOCATION	DRILL MACHINE	DEPTH	DATE	
B-12	45+73 3 ft Left 1	CME 55 TRUCK		6/29/2022	
ELEVATION (ft)	NORTHING (ft)	DRILLER	SURFACE WATER DEPTH (ft)		
373.7	735376	Campbell, T.	N/A		
TOTAL DEPTH (ft)	EASTING (ft)	DRILL METHOD	HAMMER TYPE		
10.0	2078801	Hollow Augers	Automatic		
DATE STARTED	DATE COMPLETED				
6/29/2022	6/29/2022				



Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM

O:\TEST BORING LOG G22029.00.GPJ FALCON FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC					
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 50+57 18 ft Right 1					HOLE	Dry	FIAD					
BORING NO. B-13		ELEVATION (ft) 376.1			NORTHING (ft) 735312		DRILL MACHINE CME 55 TRUCK							
TOTAL DEPTH (ft) 10.0		DATE STARTED 6/29/2022			DATE COMPLETED 6/29/2022		DRILL METHOD Hollow Augers							
EASTING (ft) 2079195		DRILLER Campbell, T.			SURFACE WATER DEPTH (ft) N/A									
DATE STARTED 6/29/2022		DATE COMPLETED 6/29/2022			DRILL METHOD Hollow Augers			HAMMER TYPE Automatic						
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB NO. DEPTH LOG	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80					100
375	1.0	2	4	8							19.2%		BITUMINOUS CONCRETE 3 inches	0.0 0.3 0.7
	3.5	4	9	12							Moist		AGGREGATE BASE COURSE 5 inches	
370	6.0	5	5	6							Moist		RESIDUUM Red, LEAN CLAY (CL)	3.0
	8.5	4	5	7							Dry		Red Tan and Gray, Sandy, LEAN CLAY (CL)	
													Gray, Sandy, SILT (ML)	8.0
365														10.0
360														
355														
Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM														

01 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



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TEST BORING LOG

PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC						
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 22+98 20 ft Left 2					HOLE	Dry	FIAD						
BORING NO. B-14		ELEVATION (ft) 382.9			NORTHING (ft) 735351		DRILL MACHINE CME 55 TRUCK								
TOTAL DEPTH (ft) 10.0		DATE STARTED 6/29/2022			DATE COMPLETED 6/29/2022		SURFACE WATER DEPTH (ft) N/A								
EASTING (ft) 2079595		DRILLER Campbell, T.			DRILL METHOD Hollow Augers			HAMMER TYPE Automatic							
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB. NO. DEPTH LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80						100
	1.0	3	5	7							Moist		382.9		0.0
	3.0	8	11	14									382.6	BITUMINOUS CONCRETE 3 inches	0.3
	3.5												382.3	AGGREGATE BASE COURSE 3 inches	0.6
380	6.0	3	9	17							15.6%	SS-12		RESIDUUM Red, Sandy, LEAN CLAY (CL) with Rock Fragments	
	8.5	6	11	19							Dry		377.4	Red - Orange and Brown, Sandy, SILT (ML) with Mica, Rock Fragments	5.5
375											Dry			Cave-in	
											Dry		372.9	Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM	10.0
370															
365															
360															

01 TEST BORING LOG G22029.00.GPJ FALCON FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC					
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 17+60 1 ft Right 2					HOLE	Dry	FIAD					
BORING NO. B-15		ELEVATION (ft) 389.9			NORTHING (ft) 735668		DRILL MACHINE CME 55 TRUCK							
TOTAL DEPTH (ft) 10.0		DATE STARTED 6/29/2022			DATE COMPLETED 6/29/2022		DRILL METHOD Hollow Augers		SURFACE WATER DEPTH (ft) N/A					
DATE STARTED 6/29/2022		DATE COMPLETED 6/29/2022			DRILL METHOD Hollow Augers		HAMMER TYPE Automatic							
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB NO. DEPTH LOG	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80					100
	1.0	4	5	5							12.4%	SS-13	389.9 389.5 BITUMINOUS CONCRETE 389.2 5 inches AGGREGATE BASE COURSE 4 inches RESIDUUM Gray to Tan, Sandy, LEAN CLAY (CL) trace Rock Fragments	0.0 0.4 0.7
	3.5	4	4	9							Moist			
385	6.0	4	6	9							Moist		384.4 Red - Tan and Gray, LEAN CLAY (CL)	5.5
	8.5	3	5	7							Dry		381.9 Red - Tan and Gray, Sandy, SILT (ML)	8.0
380													379.9	10.0
Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM														

01 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina		LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC							
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING NO. B-16		BORING LOCATION 13+68 0 ft Left 2		HOLE	Dry	FIAD							
ELEVATION (ft) 408.3		NORTHING (ft) 736059		DRILL MACHINE CME 55 TRUCK		DEPTH									
TOTAL DEPTH (ft) 10.0		EASTING (ft) 2079806		DRILLER Campbell, T.		DATE	6/29/2022								
DATE STARTED 6/29/2022		DATE COMPLETED 6/29/2022		DRILL METHOD Hollow Augers		SURFACE WATER DEPTH (ft) N/A									
						HAMMER TYPE Automatic									
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB NO. DEPTH	LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS	Depth (ft)
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80						
408.3	0.0												408.3		0.0
408.0	0.3												408.0	BITUMINOUS CONCRETE	0.3
407.7	0.6												407.7	4 inches	0.6
														AGGREGATE BASE COURSE	
														4 inches	
														RESIDUUM	
														Brown, Sandy, LEAN CLAY (CL)	
405	3.5	6	22	17									405.3	Red, CLAYEY GRAVEL (GC)	3.0
										8.6%	SS-14				
402.8	6.0	15	8	10									402.8	Red, Sandy, LEAN CLAY (CL) with Rock Fragments	5.5
400	8.5	3	7	9									400.3	Red, SILT (ML) Saprolitic	8.0
398.3	10.0												398.3		10.0
Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM															

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina		LOGGED BY Hill, J.		GROUND WATER	0 HOUR	STATIC					
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 9+38 1 ft Right 2				HOLE	Dry	FIAD					
BORING NO. B-17		BORING LOCATION 9+38 1 ft Right 2				DEPTH							
ELEVATION (ft) 420.6		NORTHING (ft) 736397		DRILL MACHINE CME 55 TRUCK		DATE	6/29/2022						
TOTAL DEPTH (ft) 10.0		EASTING (ft) 2079577		DRILLER Campbell, T.		SURFACE WATER DEPTH (ft) N/A							
DATE STARTED 6/29/2022		DATE COMPLETED 6/29/2022		DRILL METHOD Hollow Augers		HAMMER TYPE Automatic							
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT			MOISTURE	LAB NO. DEPTH LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80	100			
420	1.0	6	7	12								0.0	
											BITUMINOUS CONCRETE 4 inches	0.3	
											AGGREGATE BASE COURSE 4 inches	0.7	
	3.5	3	11	34		19				14.4%	RESIDUUM Red, Sandy, LEAN CLAY (CL) with Quartz Fragments		
										Moist			
415	6.0	4	8	11		45					Red, Sandy, SILT (ML) with Quartz Fragments	5.5	
										Dry		Cave-in	
	8.5	4	8	12		19				Dry			
										Dry			
						20							
410											Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM	10.0	

01 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

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TEST BORING LOG

PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Lane, R.		GROUND WATER	O HOUR	STATIC					
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING NO. B-18		BORING LOCATION 5+36 1 ft Left 2		HOLE	Dry	FIAD						
ELEVATION (ft) 433.0		NORTHING (ft) 736764		DRILL MACHINE CME 55 TRUCK		DEPTH								
TOTAL DEPTH (ft) 10.0		EASTING (ft) 2079430		DRILLER Campbell, T.		DATE 6/29/2022		SURFACE WATER DEPTH (ft) N/A						
DATE STARTED 6/29/2022			DATE COMPLETED 6/29/2022			DRILL METHOD Hollow Augers		HAMMER TYPE Automatic						
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SOIL AND ROCK DESCRIPTION GROUNDWATER READINGS				
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80	100	MOISTURE	LAB NO. DEPTH LOG	Elev. (ft)	Depth (ft)
	1.0		4	9	11								433.0	0.0
	3.5	3	6	6		20					Moist		432.7	0.3
	6.0	5	9	11		12							432.4	0.6
	8.5	5	9	14		20					17.3%	SS-16		
						23					Dry		427.5	5.5

O1 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM

Vertical Scale: 1"=3'



PROJECT NO. G22029.00		PROJECT LOCATION Raleigh, North Carolina			LOGGED BY Lane, R.		GROUND WATER	0 HOUR	STATIC							
PROJECT NAME Buck Jones Rd Waterline Replacements		BORING LOCATION 2+20 9 ft Left 2					HOLE	Dry	FIAD							
BORING NO. B-19		ELEVATION (ft) 438.2			NORTHING (ft) 737056		DRILL MACHINE CME 55 TRUCK		DEPTH							
TOTAL DEPTH (ft) 10.0		DATE STARTED 6/29/2022			DATE COMPLETED 6/29/2022		DRILLER Campbell, T.		SURFACE WATER DEPTH (ft) N/A							
DATE STARTED 6/29/2022		DATE COMPLETED 6/29/2022			DRILL METHOD Hollow Augers		HAMMER TYPE Automatic									
ELEV. (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					MOISTURE	LAB NO. DEPTH	LOG	Elev. (ft)	SOIL AND ROCK DESCRIPTION	Depth (ft)	
		0.5 ft	0.5 ft	0.5 ft	0	20	40	60	80	100						
438.2	0.0												438.2		0.0	
437.7	0.5												437.7	BITUMINOUS CONCRETE	0.5	
437.4	0.8												437.4	6 inches	0.8	
														AGGREGATE BASE COURSE		
														4 inches		
														RESIDUUM		
														Red, PLASTIC CLAY (CH) with Sand		
435.2	3.0												435.2	Tan to White, Sandy, SILT (ML) Saprolitic	3.0	
432.7	5.5												432.7	Tan, Sandy, SILT (ML)	5.5	
428.2	10.0												428.2		10.0	
														Boring Terminated at 10.0 feet Below Current Ground Surface in RESIDUUM		

01 TEST BORING LOG G22029.00.GPJ FALCON_FORMAT.GDT 7/14/22

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SUMMARY OF SUBSURFACE MATERIALS

Project No.: G22029.00

Project Name: Buck Jones Rd Waterline Replacements

Project Location: Raleigh, North Carolina

Boring			Surface Thickness (in)				Fill Thickness (ft)	Alluvium Thickness (ft)	Coastal Deposit		Residuum		PWR		Rock		Groundwater			
ID	Depth	Elev.	Asphalt		Agg. Base				Depth (ft)	Elev. (ft)	Depth (ft)	Elev. (ft)	Depth (ft)	Elev. (ft)	Depth (ft)	Elev. (ft)	0-hour		24-hour	
															Depth	Elev.	Depth	Elev.		
B-01	10.0	415.6	2		7						0.8	414.8					Dry	---	FIAD	---
B-02	8.0	425.0	2		8						0.8	424.2					Dry	---	FIAD	---
B-03	10.0	410.0	3		10						1.1	408.9					Dry	---	FIAD	---
B-04	10.0	410.4	2		8						0.8	409.6					Dry	---	FIAD	---
B-05	10.0	403.9	3		9						1.0	402.9					Dry	---	FIAD	---
B-06	10.0	382.6	5		16		8.3										Dry	---	FIAD	---
B-07	10.0	412.8	4		5						0.7	412.1					Dry	---	FIAD	---
B-08	10.0	430.8	5		6						0.9	429.9					Dry	---	FIAD	---
B-09	10.0	411.7	4		6						0.8	410.9					Dry	---	FIAD	---
B-10	10.0	395.1	6		4						0.8	394.3					Dry	---	FIAD	---
B-11	10.0	381.0	5		6		2.1				3.0	378.0					Dry	---	FIAD	---
B-12	10.0	373.8	5		5						0.8	373.0					Dry	---	FIAD	---
B-13	10.0	376.2	4		5						0.7	375.5					Dry	---	FIAD	---
B-14	10.0	383.0	4		4						0.6	382.4					Dry	---	FIAD	---
B-15	10.0	389.8	5		4						0.7	389.1					Dry	---	FIAD	---
B-16	10.0	408.3	4		4						0.6	407.7					Dry	---	FIAD	---
B-17	10.0	420.3	4		5						0.7	419.6					Dry	---	FIAD	---
B-18	10.0	432.9	4		4						0.6	432.3					Dry	---	FIAD	---
B-19	10.0	438.3	6		4						0.8	437.5					Dry	---	FIAD	---
B-20	10.0	378.7	5		5		2.2				3.0	375.7					Dry	---	FIAD	---
B-21	10.0	388.7	4		5		1.3				2.0	386.7					Dry	---	FIAD	---
B-22	10.0	403.1	4		4						0.6	402.5					Dry	---	FIAD	---
B-23	10.0	419.4	4		5						0.7	418.7					Dry	---	FIAD	---

001 SUMMARY OF SUBSURFACE MATERIALS G22029.00.GPJ FALCON FORMAT.GDT 7/8/22

Bold text denotes the origin strata of termination or refusal.

APPENDIX C

SUMMARY OF SOIL INDEX TESTING.....	C-1
LABORATORY COMPACTION CURVES.....	C-2
ATTERBERG LIMITS RESULTS.....	C-3
GRAIN SIZE DISTRIBUTION CURVES	C-4



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LABORATORY COMPACTION TEST RESULTS

Project No.: G22029.00

Project Name: Buck Jones Rd Waterline Replacements

Project Location: Raleigh, North Carolina

SPECIMEN DATA

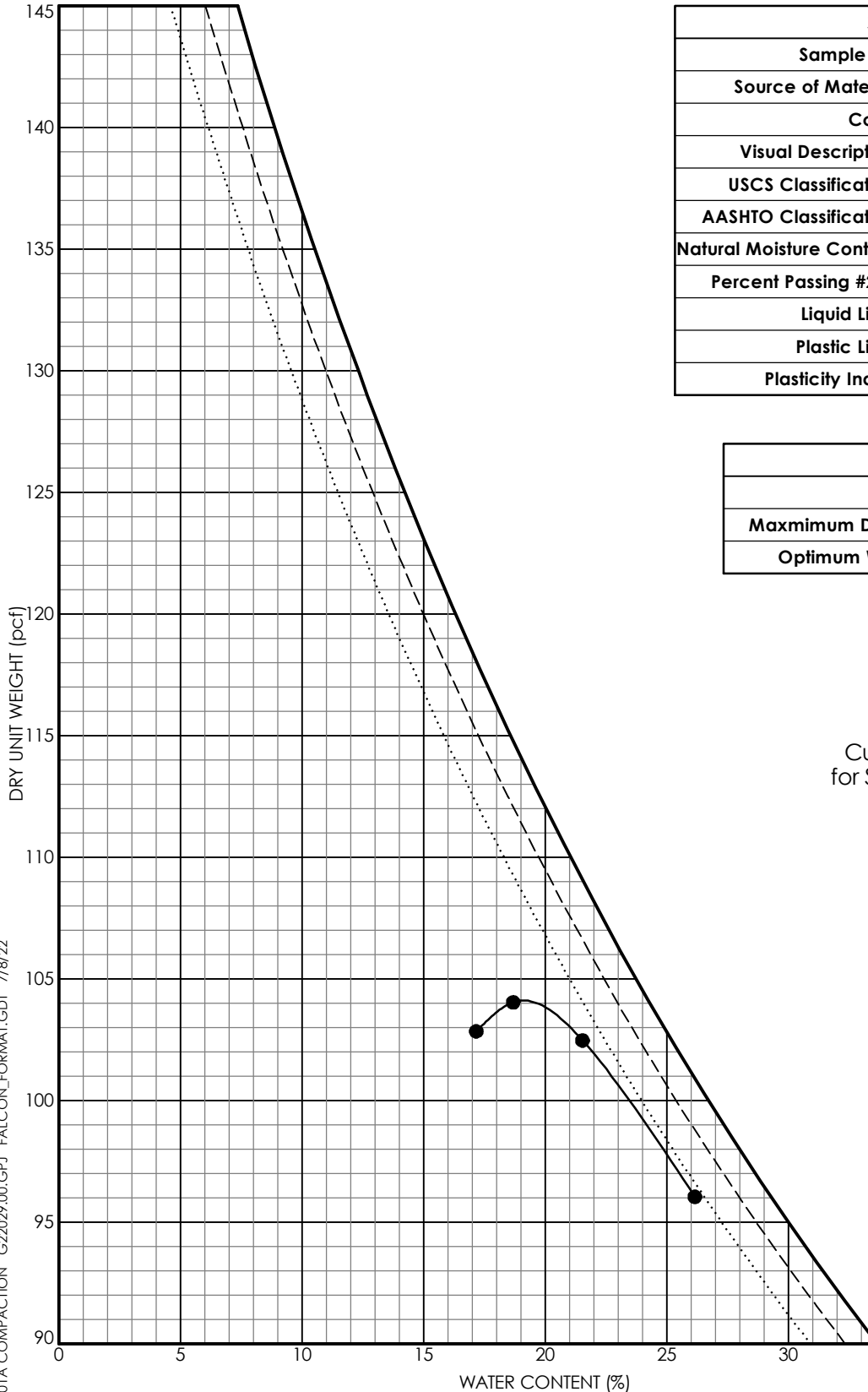
Sample No:	BS-01
Source of Material:	B-03
Color:	Weak Red
Visual Description:	
USCS Classification:	SILT with SAND(ML)
AASHTO Classification:	A-4
Natural Moisture Content:	20.3 %
Percent Passing #200:	71.6 %
Liquid Limit:	38
Plastic Limit:	38
Plasticity Index:	NP

TEST RESULTS

Test Method:	ASTM D698 Method A
Maximum Dry Unit Weight:	104.1 pcf
Optimum Water Content:	19.1 %

Curves of 100% Saturation
 for Specific Gravity Equal to:

- 2.8
- - - 2.7
- 2.6





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LABORATORY COMPACTION TEST RESULTS

Project No.: G22029.00

Project Name: Buck Jones Rd Waterline Replacements

Project Location: Raleigh, North Carolina

SPECIMEN DATA

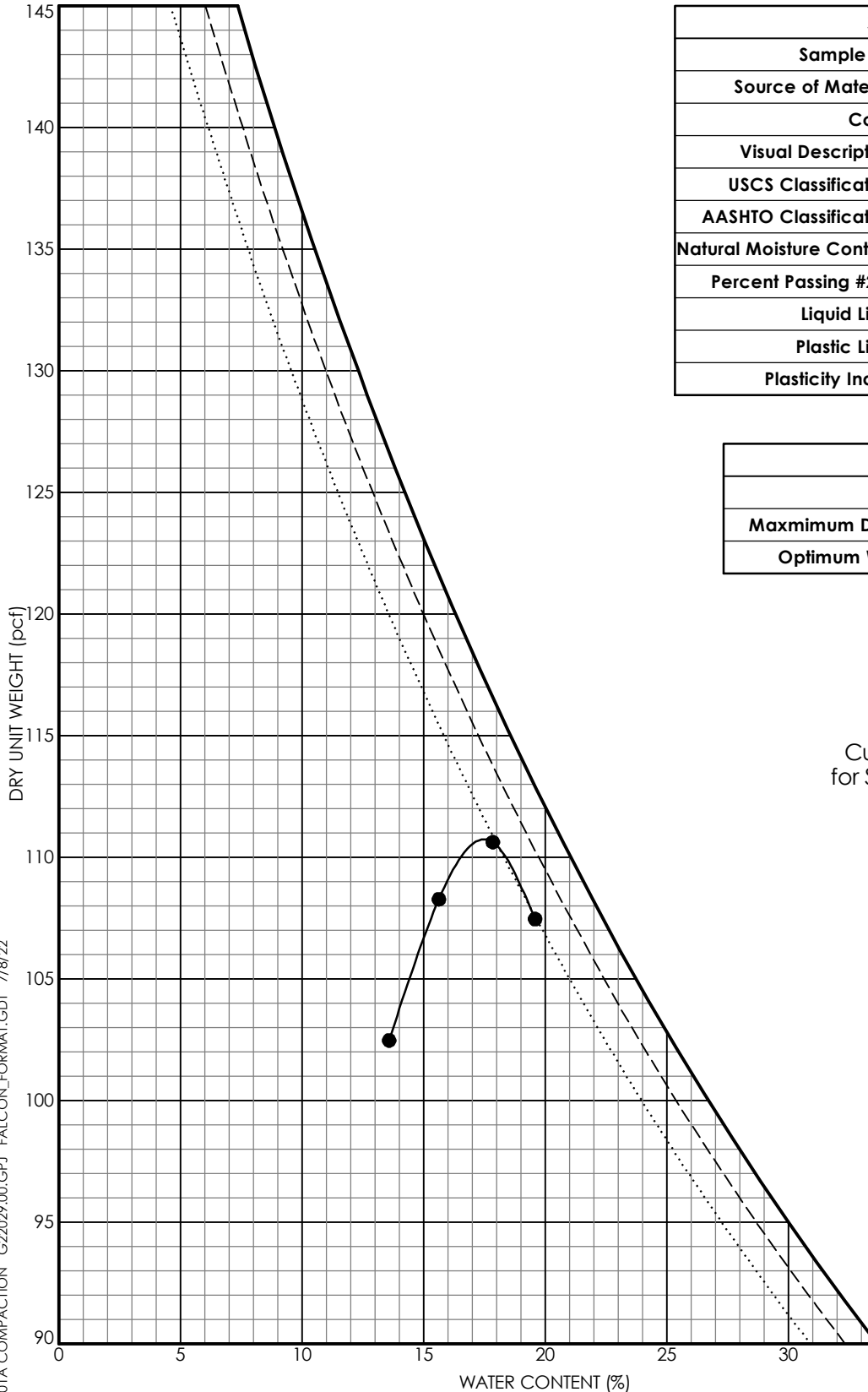
Sample No:	BS-02
Source of Material:	B-13
Color:	Reddish Brown
Visual Description:	
USCS Classification:	SANDY LEAN CLAY(CL)
AASHTO Classification:	A-7-6
Natural Moisture Content:	19.2 %
Percent Passing #200:	61.7 %
Liquid Limit:	45
Plastic Limit:	25
Plasticity Index:	20

TEST RESULTS

Test Method:	ASTM D698 Method A
Maximum Dry Unit Weight:	110.7 pcf
Optimum Water Content:	17.5 %

Curves of 100% Saturation
 for Specific Gravity Equal to:

- 2.8
- - - 2.7
- 2.6





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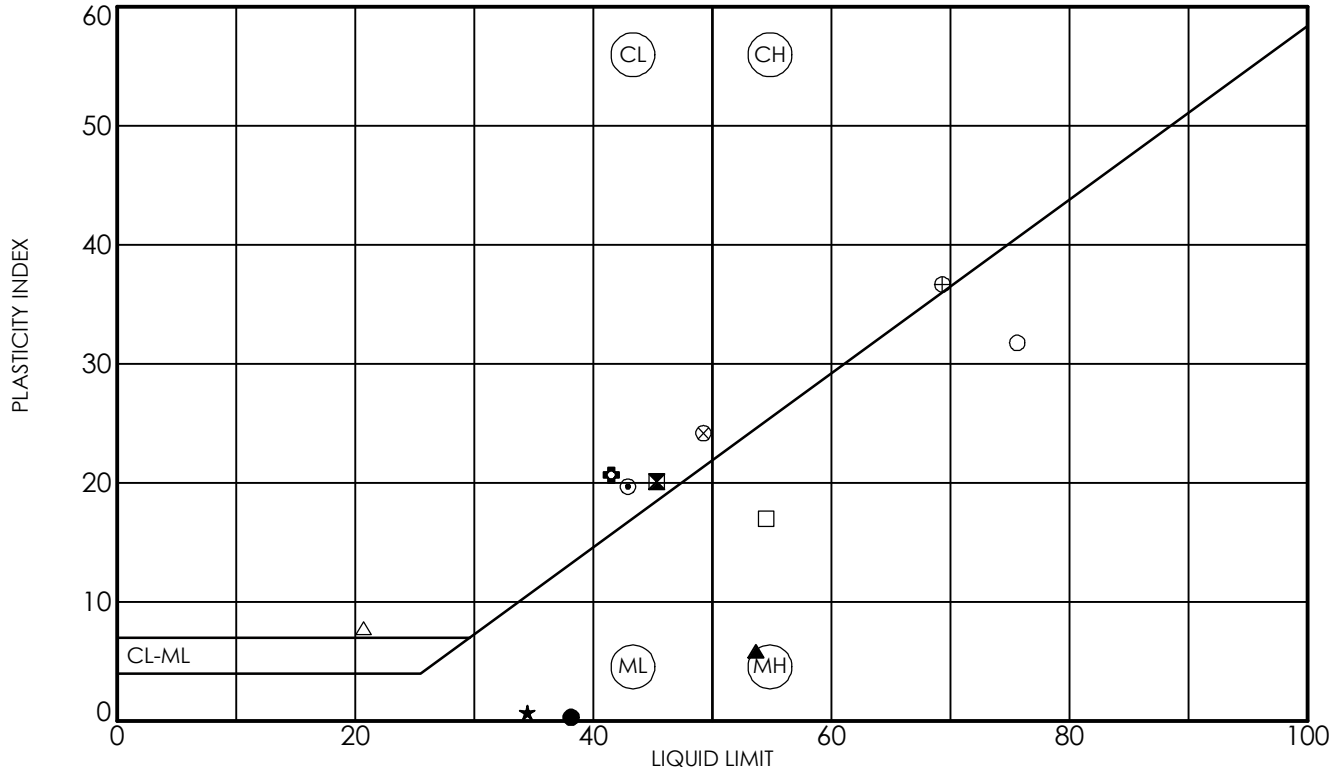
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ATTERBERG LIMITS RESULTS

Project No.: G22029.00

Project Name: Buck Jones Rd Waterline Replacements

Project Location: Raleigh, North Carolina



Sample Identification		LL	PL	PI	Fines	USCS Classification AASHTO Classification
●	BS-01 B-03	1.0 - 6.0	38	38	NP	71.6 SILT with SAND (ML) A-4
⊠	BS-02 B-13	1.0 - 5.0	45	25	20	61.7 SANDY LEAN CLAY (CL) A-7-6
▲	SS-02 B-02	1.5 - 2.0	54	48	6	70.1 ELASTIC SILT with SAND (MH) A-5
★	SS-04 B-05	3.5 - 5.0	34	34	NP	67.3 SANDY SILT (ML) A-4
⊙	SS-05 B-06	1.0 - 2.5	43	23	20	51.9 SANDY LEAN CLAY with GRAVEL (CL) A-7-6
⊞	SS-07 B-08	1.0 - 2.5	42	21	21	49.0 CLAYEY SAND with GRAVEL (SC) A-7-6
○	SS-09 B-10	1.0 - 2.5	76	44	32	76.6 ELASTIC SILT with SAND (MH) A-7-5
△	SS-13 B-15	1.0 - 2.5	21	13	8	61.9 SANDY LEAN CLAY (CL) A-4
⊗	SS-15 B-17	1.0 - 2.5	49	25	24	52.8 SANDY LEAN CLAY (CL) A-7-6
⊕	SS-17 B-19	1.0 - 2.5	69	33	36	80.8 FAT CLAY with SAND (CH) A-7-5
□	SS-20 B-22	3.5 - 5.0	55	38	17	70.6 ELASTIC SILT with SAND (MH) A-7-5

02 ATTERBERG G22029.00.GPJ FALCON FORMAT.GDT 7/8/22



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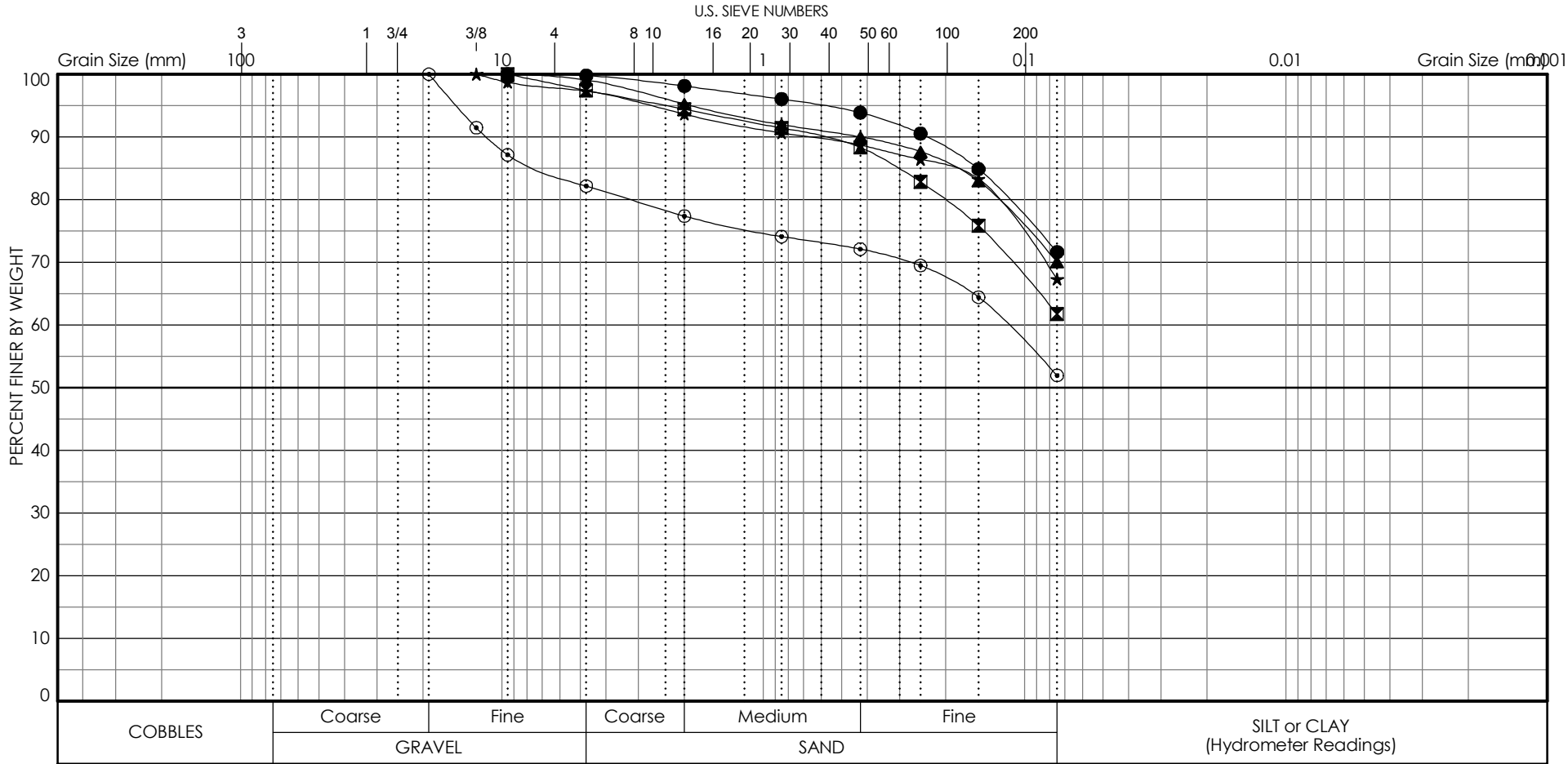
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GRAIN SIZE DISTRIBUTION

Project No.: G22029.00

Project Name: Buck Jones Rd Waterline Replacements

Project Location: Raleigh, North Carolina



03.GRAIN SIZE G22029.00.GPJ 7/8/22

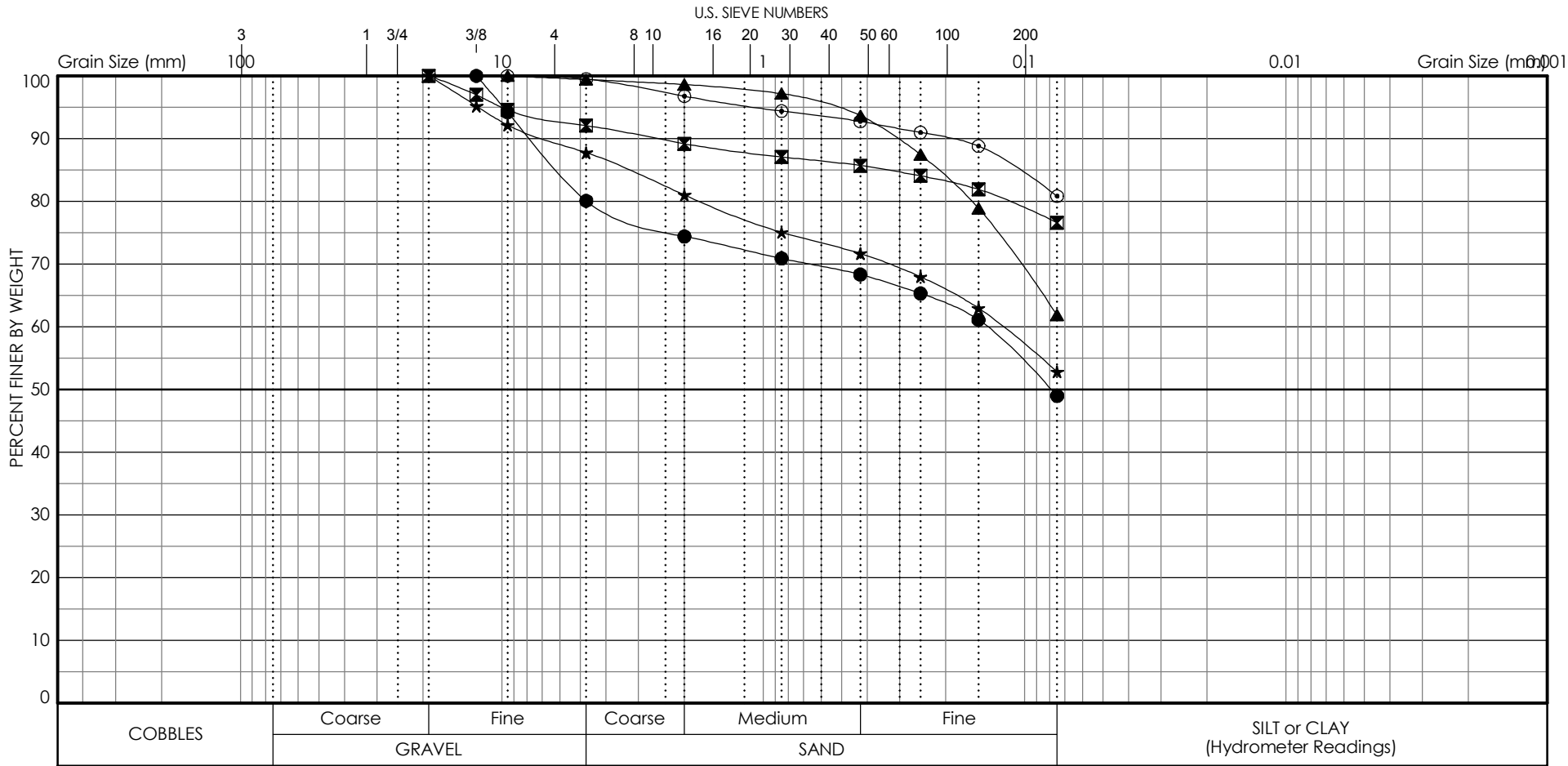
Sample ID	Source	Depth (ft)	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	C _c	C _u	%Gravel	%Sand	%Silt/%Clay	LL	PI	USCS Classification AASHTO Classification
● BS-01	B-03	1.0 - 6.0	9.5						0	28	71.6	38	NP	SILT with SAND (ML) A-4
■ BS-02	B-13	1.0 - 5.0	9.5						3	36	61.7	45	20	SANDY LEAN CLAY (CL) A-7-6
▲ SS-02	B-02	1.5 - 2.0	9.5						1	29	70.1	54	6	ELASTIC SILT with SAND (MH) A-5
★ SS-04	B-05	3.5 - 5.0	12.5						3	30	67.3	34	NP	SANDY SILT (ML) A-4
⊙ SS-05	B-06	1.0 - 2.5	19	0.117					18	30	51.9	43	20	SANDY LEAN CLAY with GRAVEL (CL) A-7-6



Project No.: G22029.00

Project Name: Buck Jones Rd Waterline Replacements

Project Location: Raleigh, North Carolina



03.GRAIN SIZE G22029.00.GPJ 7/8/22

Sample ID	Source	Depth (ft)	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	C _c	C _u	%Gravel	%Sand	%Silt/%Clay	LL	PI	USCS Classification AASHTO Classification
● SS-07	B-08	1.0 - 2.5	12.5	0.141					20	31	49.0	42	21	CLAYEY SAND with GRAVEL (SC) A-7-6
■ SS-09	B-10	1.0 - 2.5	19						8	15	76.6	76	32	ELASTIC SILT with SAND (MH) A-7-5
▲ SS-13	B-15	1.0 - 2.5	9.5						1	38	61.9	21	8	SANDY LEAN CLAY (CL) A-4
★ SS-15	B-17	1.0 - 2.5	19	0.122					12	35	52.8	49	24	SANDY LEAN CLAY (CL) A-7-6
◎ SS-17	B-19	1.0 - 2.5	9.5						0	19	80.8	69	36	FAT CLAY with SAND (CH) A-7-5

PART-3

VACUUM EXCAVATION TEST HOLE REPORT

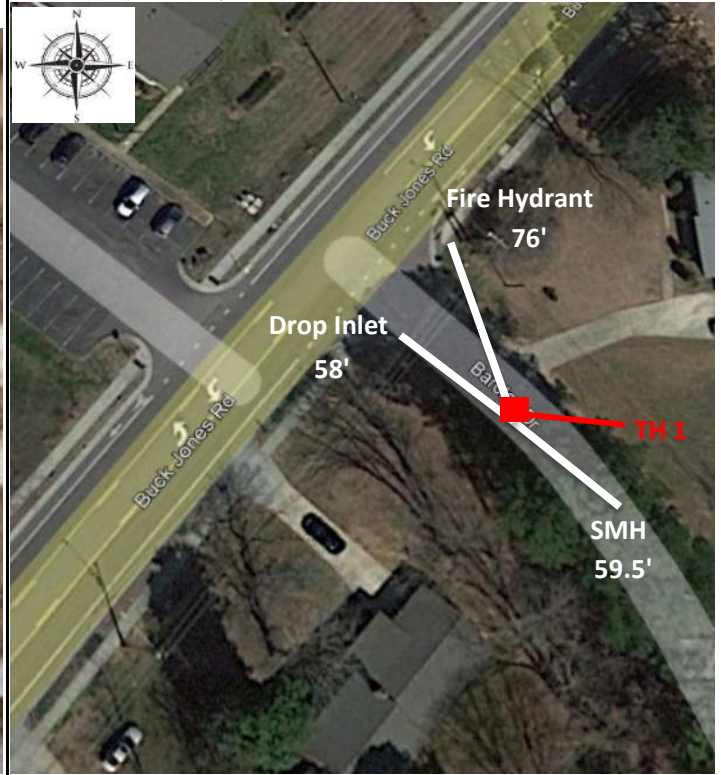
PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	1
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Barclay Rd & Buck Jones Rd (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	6/1/2022	PAVEMENT THICKNESS:	0.3'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	2.5" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Coated Steel	MATERIAL:			
DEPTH:	1.70'	DEPTH:			
DIRECTION:	N	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.



Test Hole Location Map



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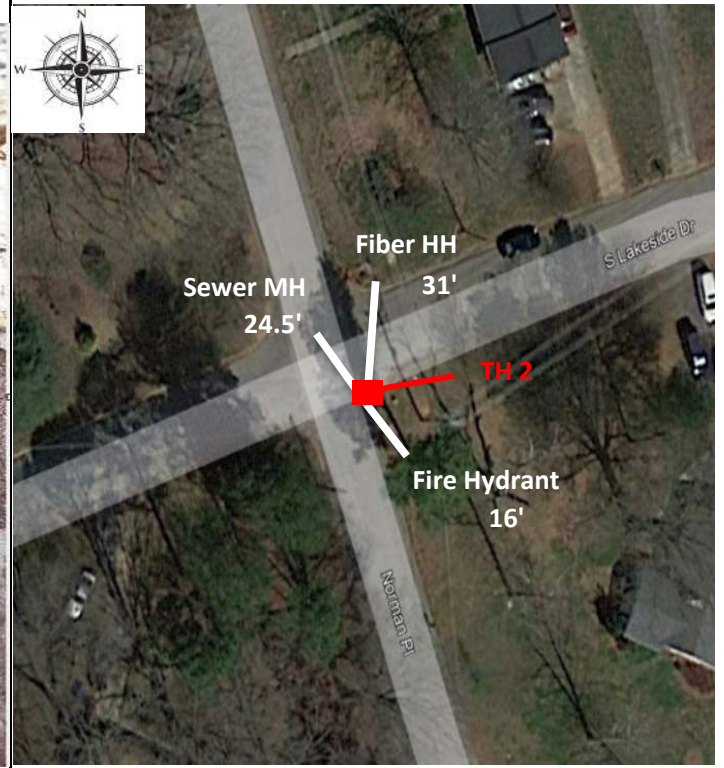
VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	2
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Lakeside & Norman (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/31/2022	PAVEMENT THICKNESS:	0.5'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Water	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	9" OD ±	SIZE:		OTHER NOTES:	Could not determine material due to corrosion. Metallic in nature.
MATERIAL:	Unknown	MATERIAL:			
DEPTH:	3.18'	DEPTH:			
DIRECTION:	N	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

Test Hole Location Map



VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	3
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Norman & Lakeside (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/31/2022	PAVEMENT THICKNESS:	0.4'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	5" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Coated Steel	MATERIAL:			
DEPTH:	2.79'	DEPTH:			
DIRECTION:	N	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

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VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	5
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Buck Jones & Norman (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	6/2/2022	PAVEMENT THICKNESS:	1.4'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	4.5" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Plastic	MATERIAL:			
DEPTH:	3.60'	DEPTH:			
DIRECTION:	E	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.



Test Hole Location Map



VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	6
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Buck Jones & Norman (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	6/2/2022	PAVEMENT THICKNESS:	1.0'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Water	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	13" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Cast Iron	MATERIAL:			
DEPTH:	4.05'	DEPTH:			
DIRECTION:	E	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.



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VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	7
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Norman & Jeffries (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	6/1/2022	PAVEMENT THICKNESS:	0.4'
UTILITY #:	1	UTILITY #:	
TYPE:	Water	TYPE:	
SIZE:	3" OD ±	SIZE:	
MATERIAL:	Unknown	MATERIAL:	
DEPTH:	3.78'	DEPTH:	
DIRECTION:	E	DIRECTION:	
UTILITY #:		UTILITY #:	
TYPE:		TYPE:	
SIZE:		SIZE:	
MATERIAL:		MATERIAL:	
DEPTH:		DEPTH:	
DIRECTION:		DIRECTION:	
		SITE PERSONNEL:	TM, NJ
		METHOD USED:	Vacuum Excavation
		OTHER NOTES:	Could not determine material due to corrosion. Metallic in nature.
		SURVEY PROVIDED BY:	
		SURVEY COORDINATES:	
		ELEVATION:	
		NORTH:	
		EAST:	

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

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VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	8
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Norman & Lakeside (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/31/2022	PAVEMENT THICKNESS:	0.6'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Water	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	7" OD ±	SIZE:		OTHER NOTES:	Could not determine material due to corrosion. Metallic in nature.
MATERIAL:	Unknown	MATERIAL:			
DEPTH:	3.34'	DEPTH:			
DIRECTION:	W	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

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VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	9
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Norman & Lakeside (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/31/2022	PAVEMENT THICKNESS:	0.3'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	2.5" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Coated Steel	MATERIAL:			
DEPTH:	2.09'	DEPTH:			
DIRECTION:	W	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

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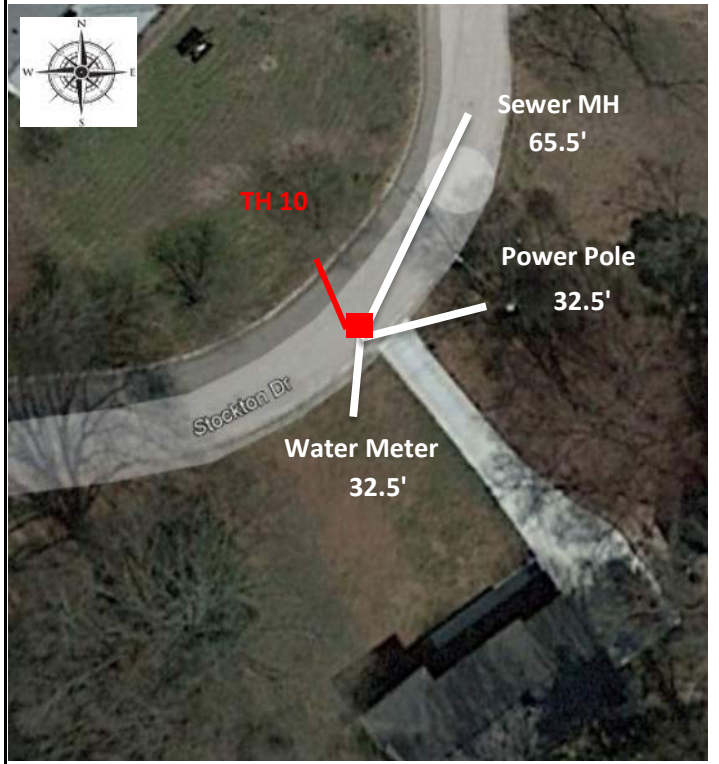
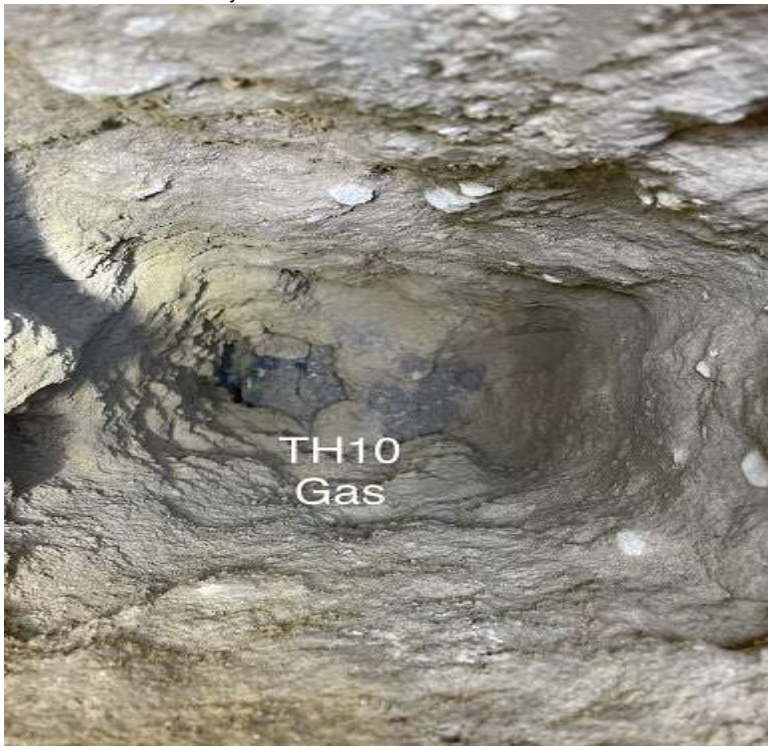
VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	10
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Stockton & Norman (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	6/1/2022	PAVEMENT THICKNESS:	0.4'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	TM, NJ
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	3" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Coated Steel	MATERIAL:			
DEPTH:	2.66'	DEPTH:			
DIRECTION:	N	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

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VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	11
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Stockton & Jeffries (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/26/2022	PAVEMENT THICKNESS:	0.6'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	CJH, BC, RC
TYPE:	Water	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	9" OD ±	SIZE:		OTHER NOTES:	Could not determine material due to corrosion. Metallic in nature.
MATERIAL:	Unknown	MATERIAL:			
DEPTH:	3.55'	DEPTH:			
DIRECTION:	S	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

Test Hole Location Map

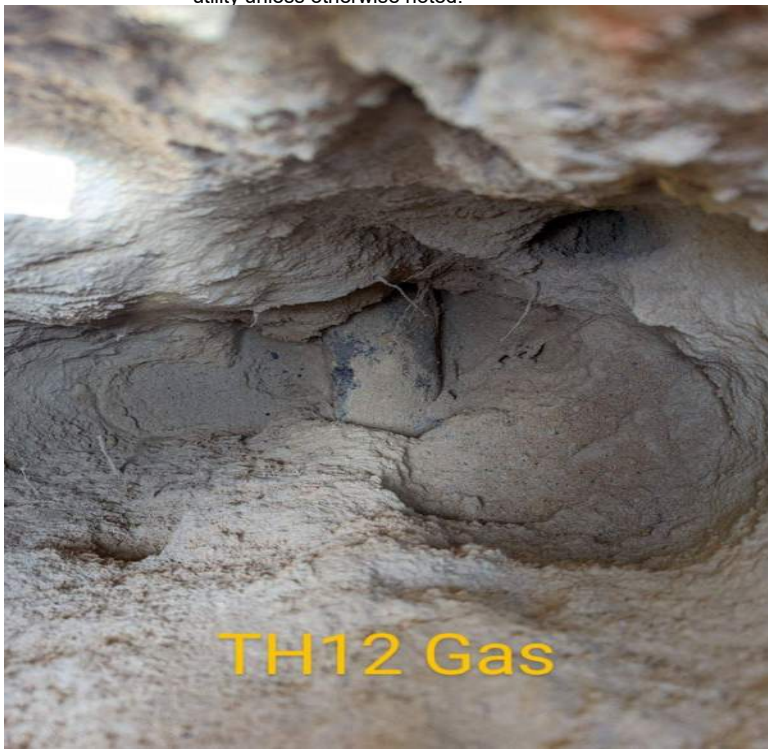


VACUUM EXCAVATION TEST HOLE REPORT

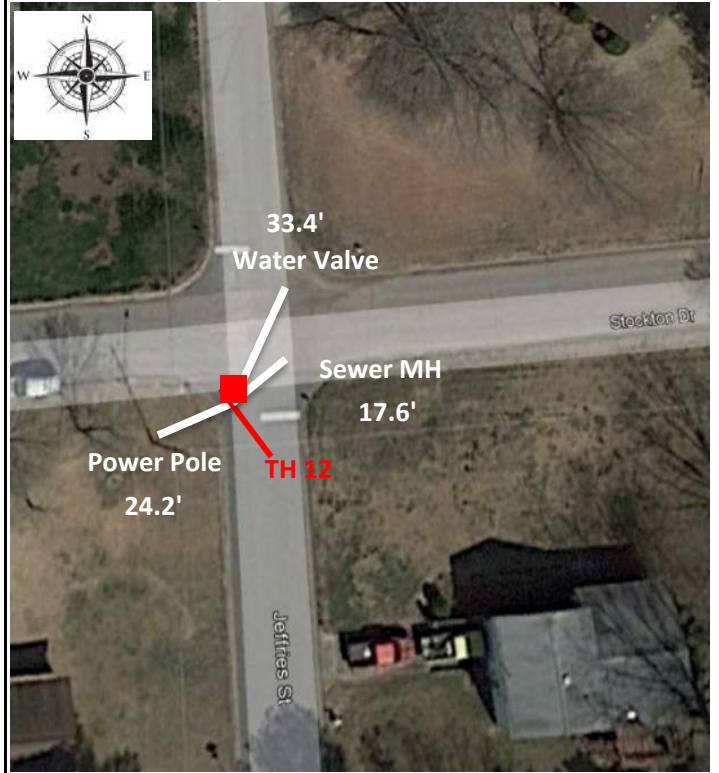
PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	12
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Stockton & Jeffries (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/26/2022	PAVEMENT THICKNESS:	0.45'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	CJH, BC, RC
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	4.5" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Steel	MATERIAL:			
DEPTH:	3.26'	DEPTH:			
DIRECTION:	S	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

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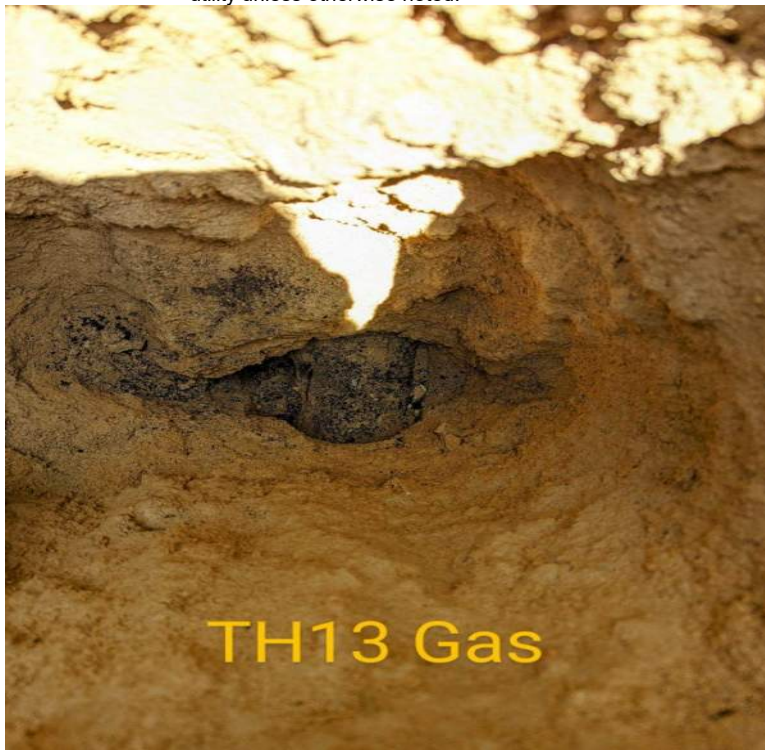
VACUUM EXCAVATION TEST HOLE REPORT

PROJECT NUMBER:	CJSC00421	TEST HOLE NUMBER:	13
CLIENT NAME:	CJS Conveyance	SOIL TYPE:	Clay
PROJECT TITLE:	Buck Jones Test Holes	SURFACE MATERIAL:	Pavement
LOCATION:	Stockton & Jeffries (Cary, NC)	PAVEMENT TYPE:	Asphalt
DATE:	5/26/2022	PAVEMENT THICKNESS:	0.4'

UTILITY #:	1	UTILITY #:		SITE PERSONNEL:	CJH, BC, RC
TYPE:	Gas	TYPE:		METHOD USED:	Vacuum Excavation
SIZE:	4.5" OD ±	SIZE:		OTHER NOTES:	
MATERIAL:	Steel	MATERIAL:			
DEPTH:	2.58'	DEPTH:			
DIRECTION:	E	DIRECTION:			
UTILITY #:		UTILITY #:		SURVEY PROVIDED BY:	
TYPE:		TYPE:		SURVEY COORDINATES:	
SIZE:		SIZE:		ELEVATION:	
MATERIAL:		MATERIAL:		NORTH:	
DEPTH:		DEPTH:		EAST:	
DIRECTION:		DIRECTION:			

Notes: All measurements obtained from top/center of associated utility unless otherwise noted.

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